PO Box 12327
Murfreesboro, TN. 37129
615-907-1240

American Steel Buildings

BUILDING LOADS / DESCRIPTION:

BUILDING:

WIDTH: 75'-0" LENGTH: 125'-0" HEIGHT: 20'-0"/20'-0"
(BUILDING DIMENSIONS ARE NOMINAL. REFER TO PLANS).

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY : IBC 21 / IBC 21

THE CONTRACTOR IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

OCCUPANCY CATEGORY:	II – Normal	
ROOF DEAD LOAD (PANELS & PURLINS):	2.50	PSF
COLLATERAL LOAD:	5.00	PSF
ROOF LIVE LOAD:	20.00	PSF (Reducible)
GROUND SNOW LOAD:	5	PSF
SNOW LOAD IMPORTANCE:	1.0000	PSF
ROOF SNOW LOAD:	3.50	PSF
SNOW EXPOSURE:	1.0000	
THERMAL FACTOR:	1.00	
BASIC WIND SPEED:	130 mph	
WIND EXPOSURE:	C	
WIND LOAD IMPORTANCE:	1.00	
INTERNAL PRESSURE COEFF.:	0.18 / −0.18	
SEISMIC IMPORTANCE FACTOR:	1.00	
SEISMIC DESIGN CATEGORY:	A	
SEISMIC ZONE:	A	
SITE CLASS:	D	
MAPPED SPECTRAL RESPONSE ACC. Ss	0.07	
S1	0.04	
SPECTRAL RESPONSE COEFF. Sds	0.07	
Sd1	0.06	
DESIGN BASE SHEAR, V: LONGITUDINAL	2.34	
TRANSVERSE	2.38	

DEFLECTION LIMITS:

EW COL:	180
EW RAF LIVE:	180
EW RAF WIND:	180
WALL GIRT:	90
PURL LIVE:	180
PURL WIND:	150
WALL PANEL:	60
ROOF PANEL LIVE:	60
ROOF PANEL WIND:	60
RF HORIZONTAL:	60
RF VERTICAL:	180
WIND BENT:	60
RF CRANE:	100
RF SEIS:	50
WIND BENT SEIS:	50

MATERIALS : MINIMUM YIELD:

HOT ROLLED BAR	FY = 50.0000ksi MIN.
STRUCTURAL STEEL SHEET	FY = 50.0000ksi MIN.
STRUCTURAL STEEL PLATE	FY = 50.0000ksi MIN.
COLD FORMED SHAPES	FY = 57.0000ksi MIN.
WALL SHEETING	FY = 60.0000ksi MIN.
ROOF SHEETING	FY = 60.0000ksi MIN.
BOLTS	A307 & A325

THE METAL BUILDING MANUFACTURER RESERVES THE RIGHT TO SUBSTITUTE THE ABOVE MATERIALS WITH EQUAL OR BETTER MATERIAL.

BOLT TIGHTENING REQUIREMENTS:

ALL BOLTED JOINTS WITH A325 BOLTS ARE SPECIFIED AS SNUG-TIGHTENED JOINTS IS ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". PRETENSIONING METHODS, INCLUDING TURN OF THE NUT AND CALIBRATED WRENCH ARE NOT REQUIRED.

BRACING NOTE:

X-BRACING IS TO BE INSTALLED TO A TAUT CONDITION WITH ALL SLACK REMOVED. DO NOT TIGHTEN BEYOND THIS STATE.

GUTTER AND MAX DOWNSPOUT SPACING:

THE GUTTER AND DOWNSPOUNT SYSTEM PROVIDED BY ASCENT BUILDINGS, LLC IS DESIGNED TO ACCOMMODATE 10.0 IN/HR RAINFALL INTENSITY AS IT CORRESPONDS TO A 5 YEAR RECURRENCE INTERVAL. DOWNSPOUTS ARE DESIGNED TO HAVE A MAX SPACING OF 44'-0 3/8" FEET AND LOCATIONS ARE SHOWN OF THE PLANS.

PRIMARY STEEL FRAMING COATING:

ALL PRIMARY FRAMING STEEL WILL RECEIVE SSPC6 CLEANING AND ONE SHOP COAT OF GRAY OXIDE PRIMER. PRIMER IS NOT A FINISH COAT. PRIMER IS FOR TRANSIT PURPOSES ONLY.

SECONDARY STEEL FRAMING COATING:

SECONDARY FRAMING STEEL WILL BE G30 GALVANIZED WITH ACRYLLIC COATING.

DESIGN NOTE:

DESIGN IS BASED UPON THE MORE SEVERE LOADING OF EITHER THE ROOF SNOW LOAD OR THE ROOF LIVE LOAD.

BUILDER / CONTRACTOR RESPONSIBILITIES:

IT IS THE RESPONSIBILITY OF THE BUILDER/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE METAL BUILDING SYSTEM MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT. THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS THAT THE METAL BUILDING SYSTEM MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.2.1 AISC CODE OF STANDARD PRACTICES, 9TH ED.) WHERE DISCREPANCIES EXIST BETWEEN THE METAL BUILDING SYSTEM MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE 9TH ED.) DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE METAL BUILDING SYSTEM MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE METAL BUILDING SYSTEM MANUFACTURER'S ENGINEER UNLESS SPECIFICALLY INDICATED.

THE CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH THE METAL BUILDING SYSTEM MANUFACTURER "FOR CONSTRUCTION" DRAWINGS.

ALL BRACING AS SHOWN AND PROVIDED BY THE METAL BUILDING SYSTEM MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.

TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE

IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION, OR COLLISION. (SECT. 7.9.1AISC CODE OF STANDARD PRACTICE, 9TH ED.)

WARNING : IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSION EFFECTS ON THE ALUMINUM ZINC ALLOY COATING WHEN THEY ARE USED IN CONTACT WITH GALVALUME STEEL PANELS. EVEN RUN-OFF FROM COPPER FLASHING, WIRING, ORTUBING ONTO GALVALUME SHOULD BE AVOIDED.

ERECTOR NOTE: PANEL BUNDLES MUST BE HANDLED WITH CARE!!! USE A SPREADER BAR FOR HANDLING. THE METAL BUILDING SYSTEM MANUFACTURER IS NOT RESPONSIBLE FOR MATERIALS DAMAGED ONSITE. STORE PANELS WHERE MOISTURE CAN PROPERLY DRAIN. THE METAL BUILDING SYSTEM MANUFACTURER WILL NOT WARRANT PANELS THAT HAVE BEEN STORED WHERE MOISTURE CAN BE CAPTURED BETWEEN PANELS THAT ARE BUNDLED.

CORRECTION OF MINOR MISFITS IN THE FIELD IS CONSIDERED NORMAL AND IS NOT SUBJECT TO BACK CHARGE. MAJOR CORRECTIVE WORK MUST BE AUTHORIZED IN ADVANCED BY THE ENGINEERING DEPARTMENT OF THE METAL BUILDING SYSTEM MANUFACTURER. REQUEST TO PERFORM CORRECTIVE WORK MUST BE SUBMITTED IN WRITING ALONG WITH PHOTOS AND A DESCRIPTION OF THE MODIFICATION THAT IS BEING REQUESTED. NO BACK CHARGE WILL BE PAID THAT IS NOT AUTHORIZED IN ADVANCED BY THE METAL BUILDING SYSTEM MANUFACTURER.

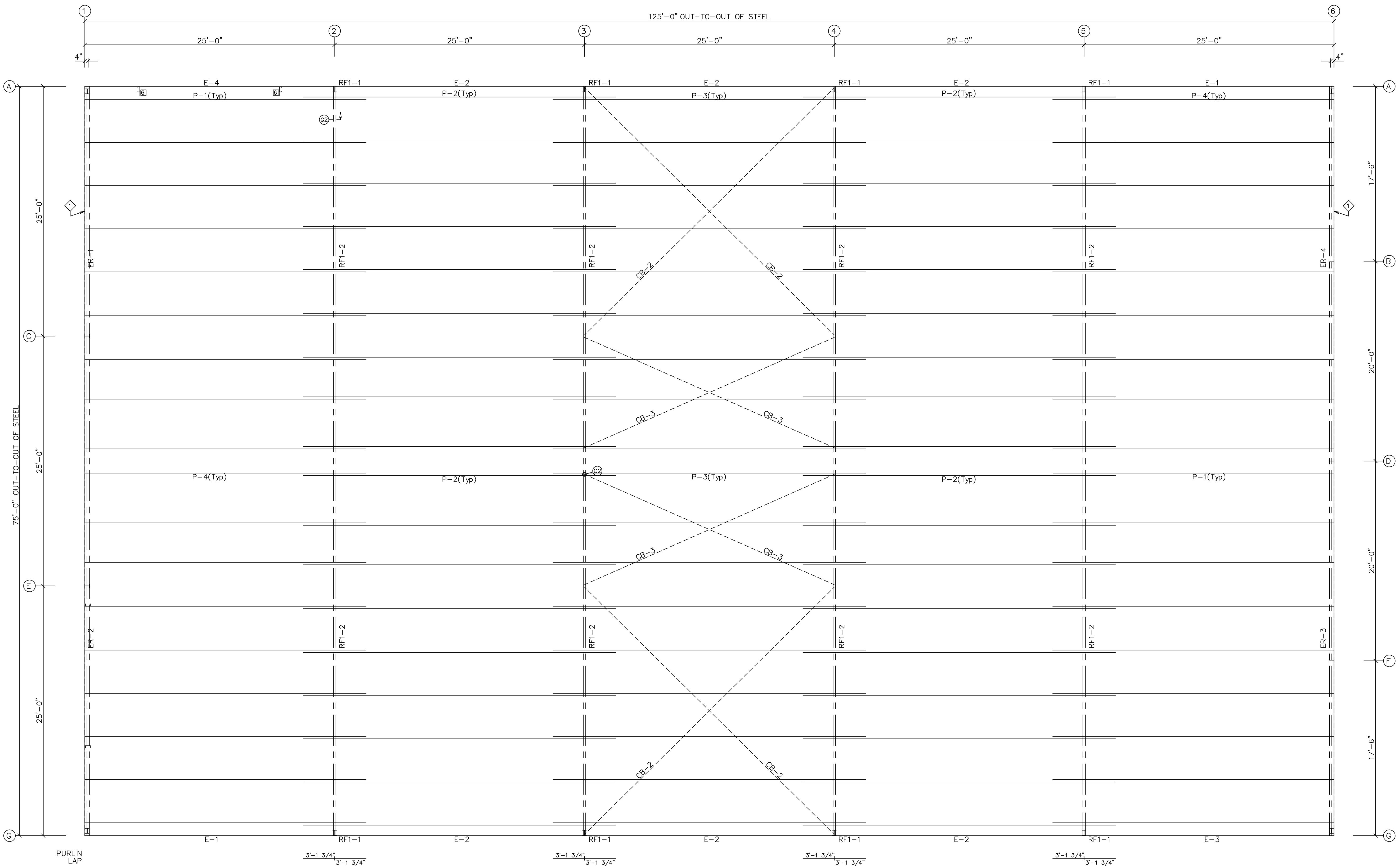
FABRICATION DRAWINGS NOTE:

THIS PROJECT HAS BEEN ORDERED FOR FABRICATION AND IS SCHEDULED OR IN PROCESS FOR FABRICATION. THESE DRAWINGS ARE PREPARED IN ACCORDANCE WITH THE ASCENT PURCHASE ORDER. CHANGES ARE NOT PERMITTED WITHOUT A CHANGE ORDER. ANY REVISIONS TO THESE DRAWINGS WILL RESULT IN POSSIBLE DELAYS AND PRICING REVISIONS. ORDER WILL BE PLACED ON HOLD IF REVISIONS ARE REQUIRED UNTIL A CHANGE ORDER IS PROCESSED AND SIGNED. A NEW FABRICATION SCHEDULED WILL BE DETERMINED AT THE TIME THE JOB IS RELEASED TO FABRICATE AND EXECUTION OF THE REVISED CHANGE ORDER.

American Steel Buildings
PO Box 12327
Murfreesboro, TN. 37129
615-907-1240

Drawing Description: Erection Drawing Cover
Customer Name: American Steel Buildings
Project Name: Ironclad Business Park - B
Project Location: Cypress, TX 77429
Job No: Q22022
Sheet No: C1 of C1

Revision	Date	Description	By	Ck'd



ROOF FRAMING PLAN

American Steel Buildings
PO Box 12327
Murfreesboro, TN, 37129
615-907-1240

Drawing Description: ROOF FRAMING PLAN

Customer Name: American Steel Buildings

Project Name: Ramirez Business Park - B

Project Location: Cypress, TX 77429

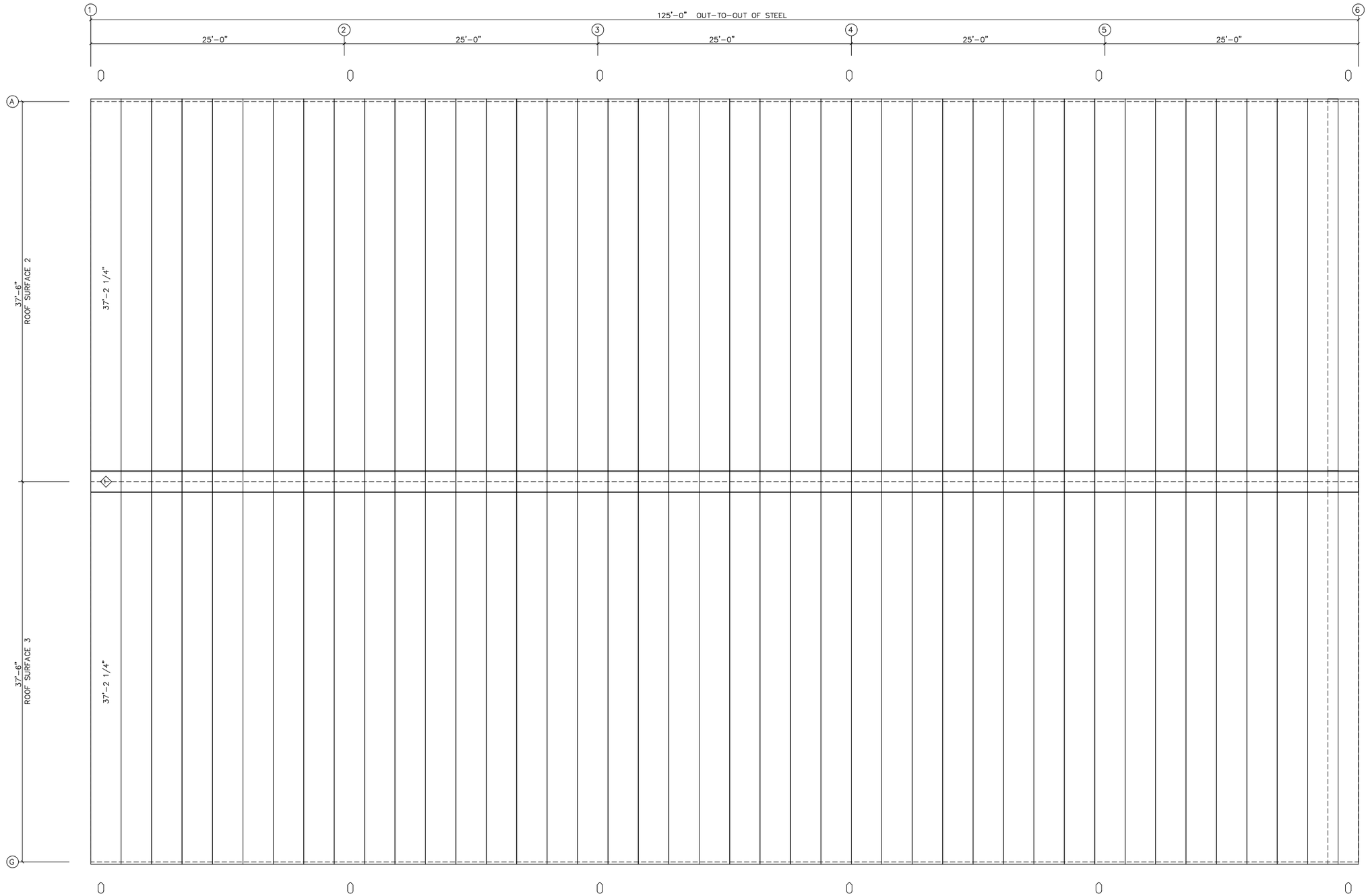
Job No: Q22022

Sheet No:

Revision	Date	Description	By	Chk'd

DOWNSPOUT LOCATIONS

Panel Table Roof Sheeting			
Quantity	Length	Part	Color
84	37'-2 1/4"	26 Ga. Rloc	Brilliant SMP Life



ROOF SHEETING PLAN

PANELS: 26 Ga. RL – Brilliant SMP Life

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Revision	Date	Description	By	Ck'd

Drawing Description: ROOF SHEETING

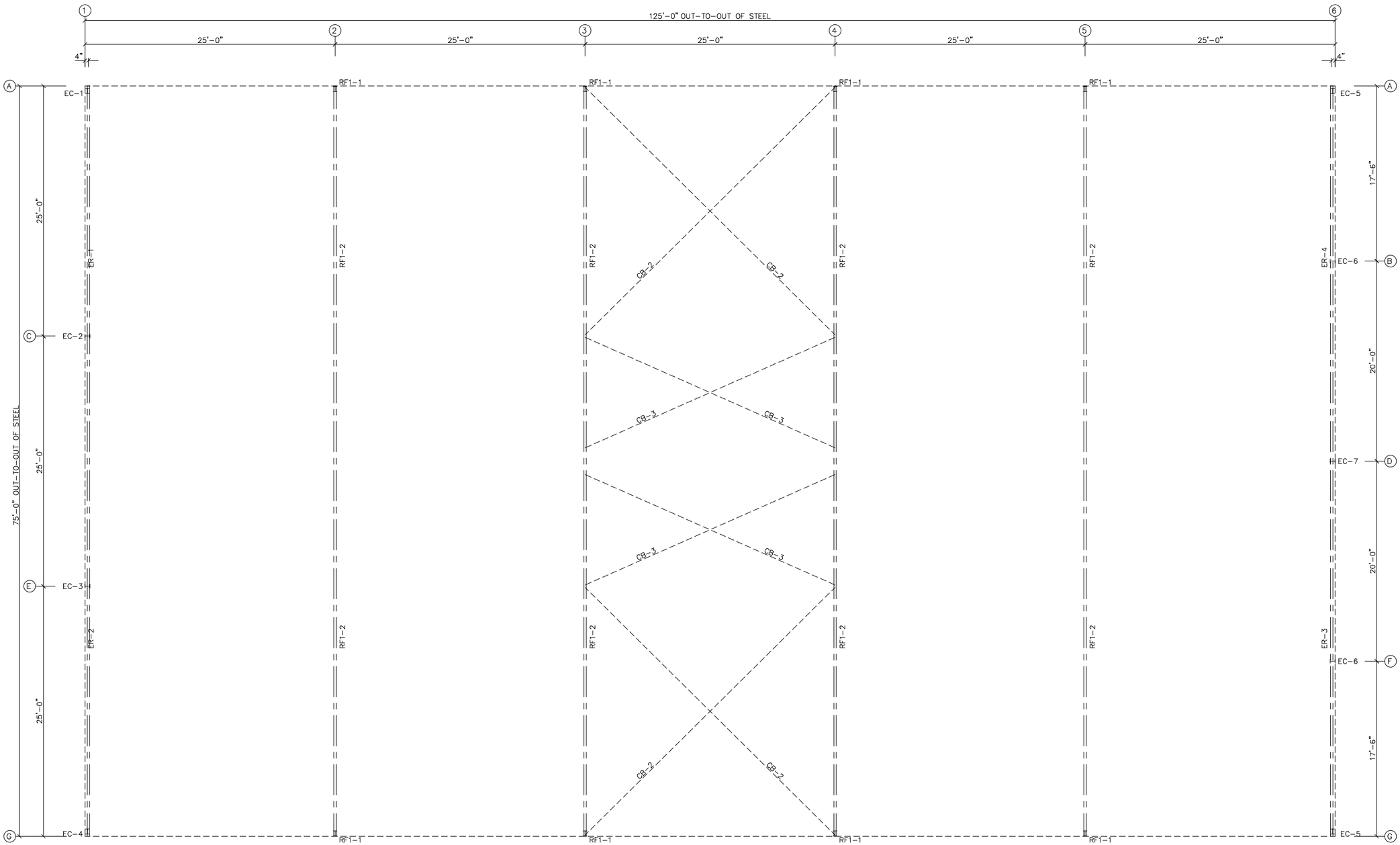
Customer Name: American Steel Buildings

Project Name: Ramirez Business Park – B

Project Location: Cypress, TX 77429

Job No: Q22022

Sheet No:



PRIMARY STEEL LAYOUT

PO Box 12327
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American Steel Buildings

PRIMARY STEEL LAYOUT

Revision	Date	Description	By	Chk'd

Drawing Description: PRIMARY STEEL LAYOUT

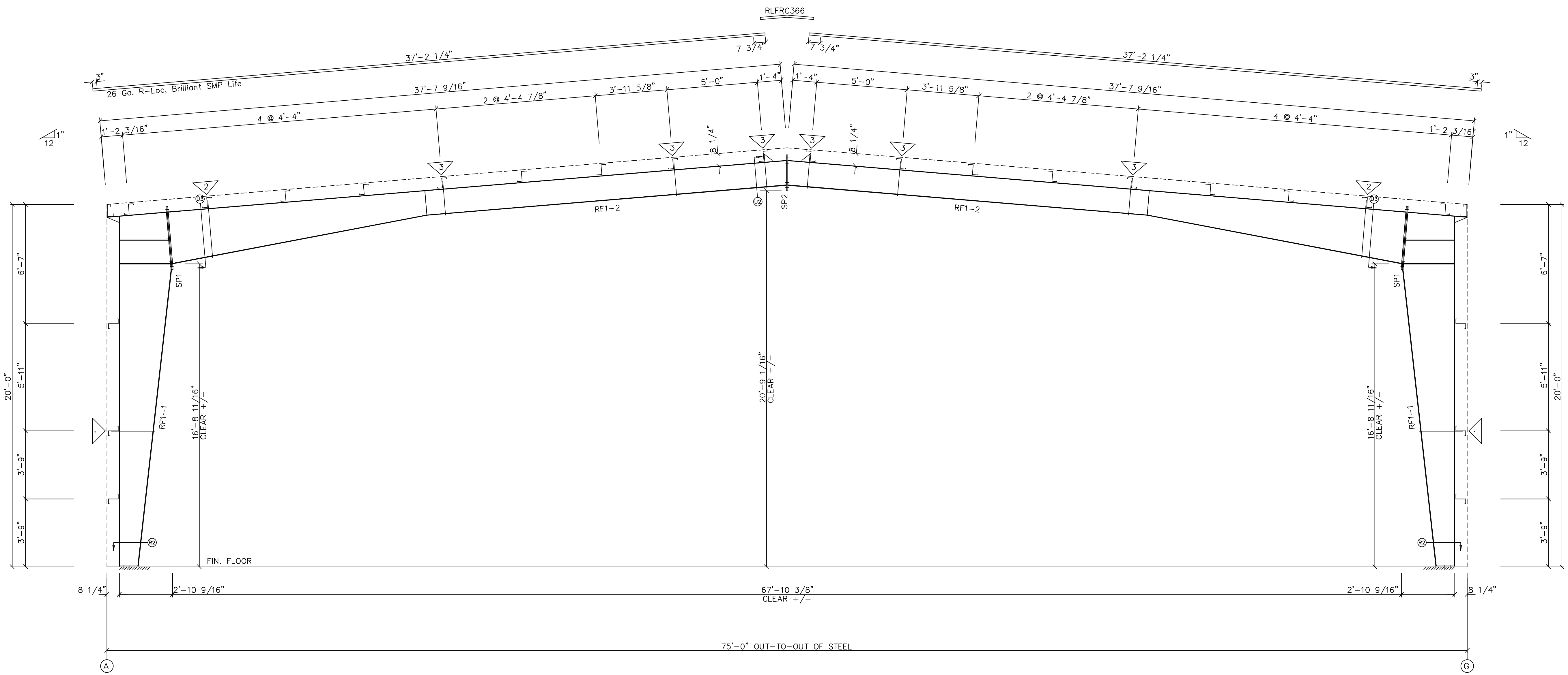
Customer Name: American Steel Buildings

Project Name: Ramirez Business Park – B

Project Location: Cypress, TX 77429

Job No: Q22022

Sheet No:



MAIN FRAME ELEVATION: FRAME LINE 2-5

American Steel Buildings

PO Box 12327
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615-907-1240

Drawing Description: CROSS SECTION

Customer Name: American Steel Buildings

Project Name: Ramirez Business Park - B

Project Location: Cypress, TX 77429

Job No: Q22022

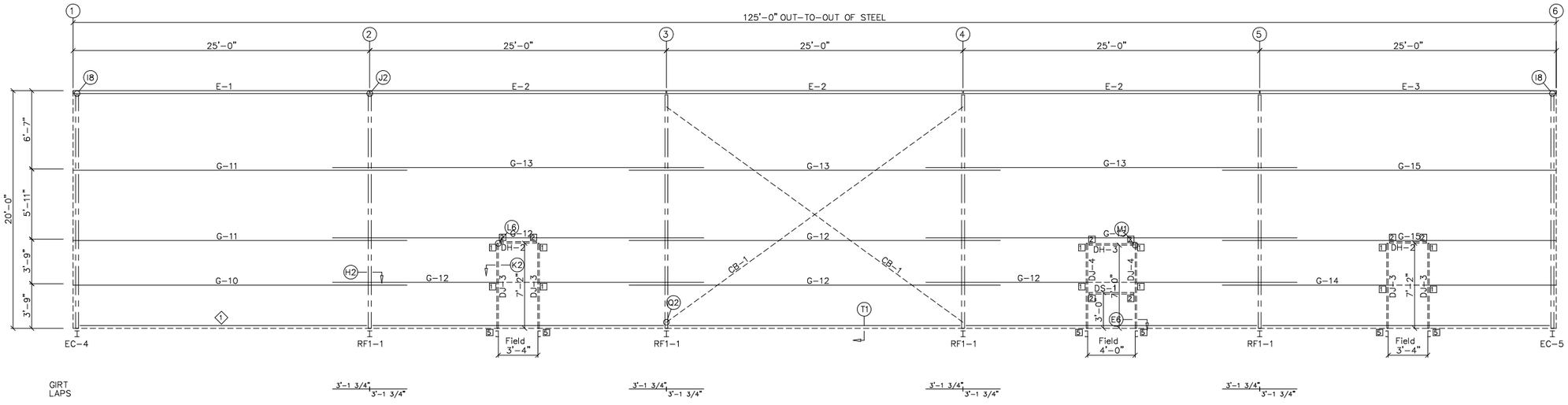
Sheet No:

Revision	Date	Description	By	Chk'd

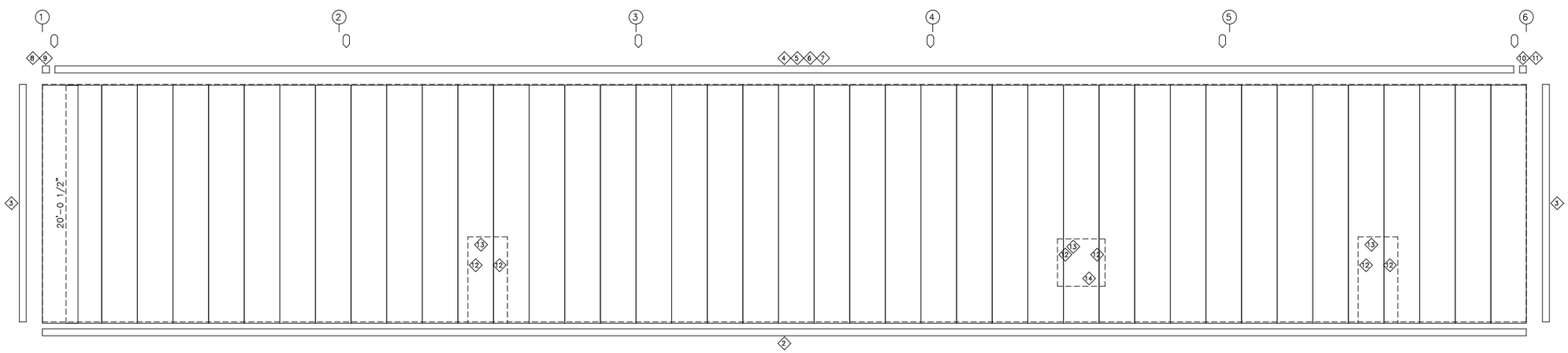
The Engineer whose seal appears hereon is an employee for the manufacturer for the material described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for the project

DOWNSPOUT LOCATIONS

Panel Table Frame Line - G			
Quantity	Length	Part	Color
42	20'-0 1/2"	26 Ga. Rloc	Gallery SMP Life



SIDEWALL FRAMING: Frame Line G



SIDEWALL SHEETING & TRIM: Frame Line G

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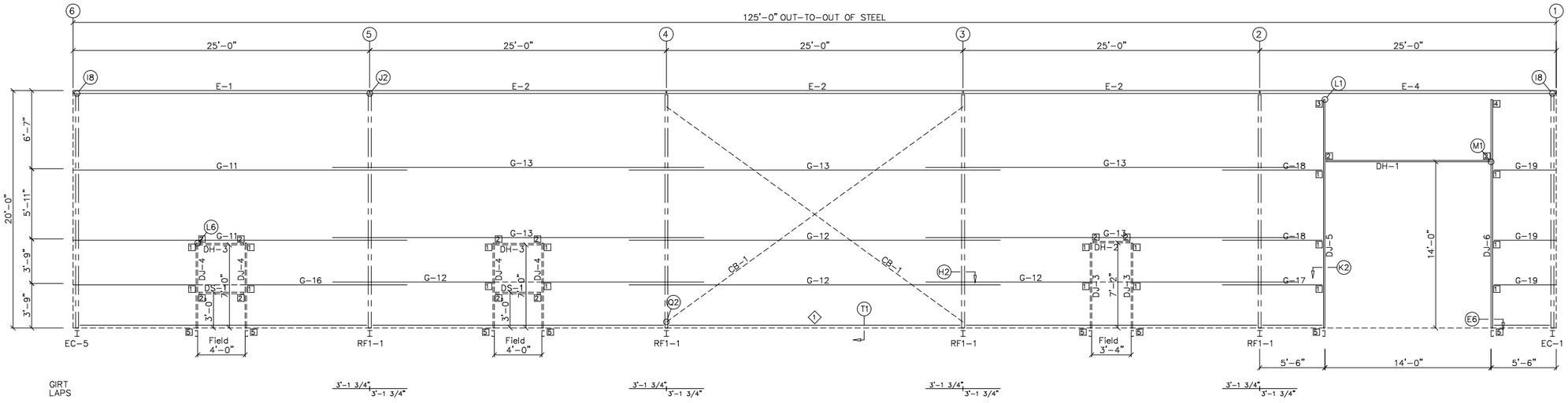
American Steel Buildings

Drawing Description: SIDEWALL ELEVATION
Sheet No:
Customer Name: American Steel Buildings
Project Name: Ramirez Business Park - B
Project Location: Cypress, TX 77429
Job No: Q22022

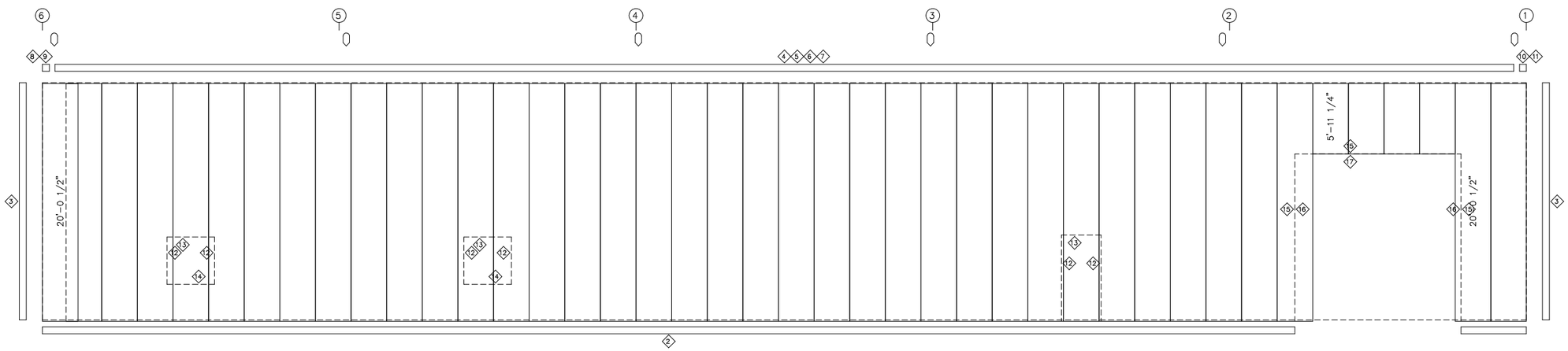
Revision	Date	Description	By	Chk'd

DOWNSPOUT LOCATIONS

Panel Table Frame Line - A			
Quantity	Length	Part	Color
38	20'-0 1/2"	26 Ga. Rloc	Gallery SMP Life
4	5'-11 1/4"	26 Ga. Rloc	Gallery SMP Life



SIDEWALL FRAMING: Frame Line A



SIDEWALL SHEETING & TRIM: Frame Line A

PO Box 12327
Murfreesboro, TN. 37129
615-907-1240
American Steel Buildings

Chk'd	By	Description	Date	Revision

Drawing Description: SIDEWALL ELEVATION

Customer Name: American Steel Buildings

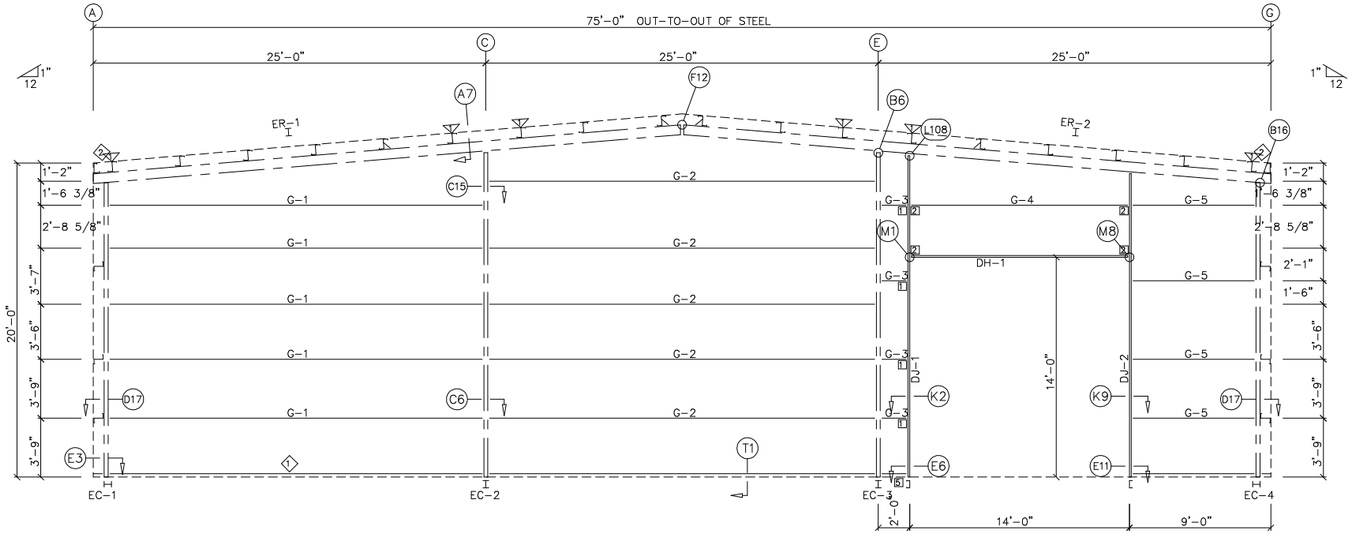
Project Name: Ramirez Business Park - B

Project Location: Cypress, TX 77429

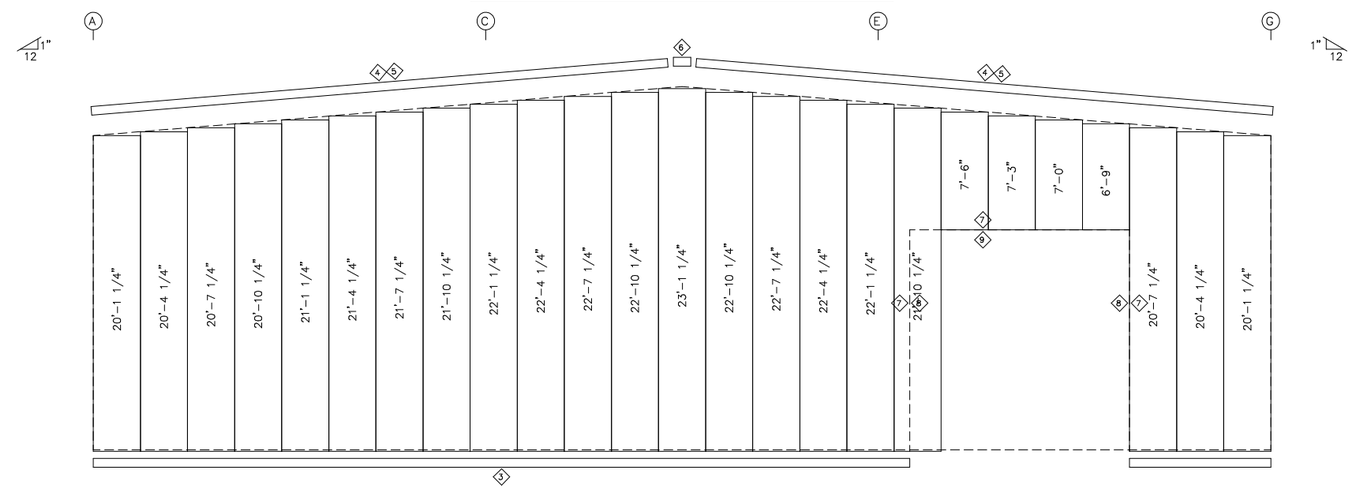
Job No: Q22022

Sheet No:

Panel Table Frame Line - 1			
Quantity	**Length**	**Part**	**Color**
2	20'-1 1/4"	26 Ga. Rloc	Gallery SMP Life
2	20'-4 1/4"	26 Ga. Rloc	Gallery SMP Life
2	20'-7 1/4"	26 Ga. Rloc	Gallery SMP Life
1	20'-10 1/4"	26 Ga. Rloc	Gallery SMP Life
1	21'-1 1/4"	26 Ga. Rloc	Gallery SMP Life
1	21'-4 1/4"	26 Ga. Rloc	Gallery SMP Life
1	21'-7 1/4"	26 Ga. Rloc	Gallery SMP Life
2	21'-10 1/4"	26 Ga. Rloc	Gallery SMP Life
2	22'-1 1/4"	26 Ga. Rloc	Gallery SMP Life
2	22'-4 1/4"	26 Ga. Rloc	Gallery SMP Life
2	22'-7 1/4"	26 Ga. Rloc	Gallery SMP Life
2	22'-10 1/4"	26 Ga. Rloc	Gallery SMP Life
1	23'-1 1/4"	26 Ga. Rloc	Gallery SMP Life
1	7'-6"	26 Ga. Rloc	Gallery SMP Life
1	7'-3"	26 Ga. Rloc	Gallery SMP Life
1	7'-0"	26 Ga. Rloc	Gallery SMP Life
1	6'-9"	26 Ga. Rloc	Gallery SMP Life



ENDWALL FRAMING: Frame Line 1



ENDWALL SHEETING & TRIM: Frame Line 1

PO Box 12327
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American Steel Buildings

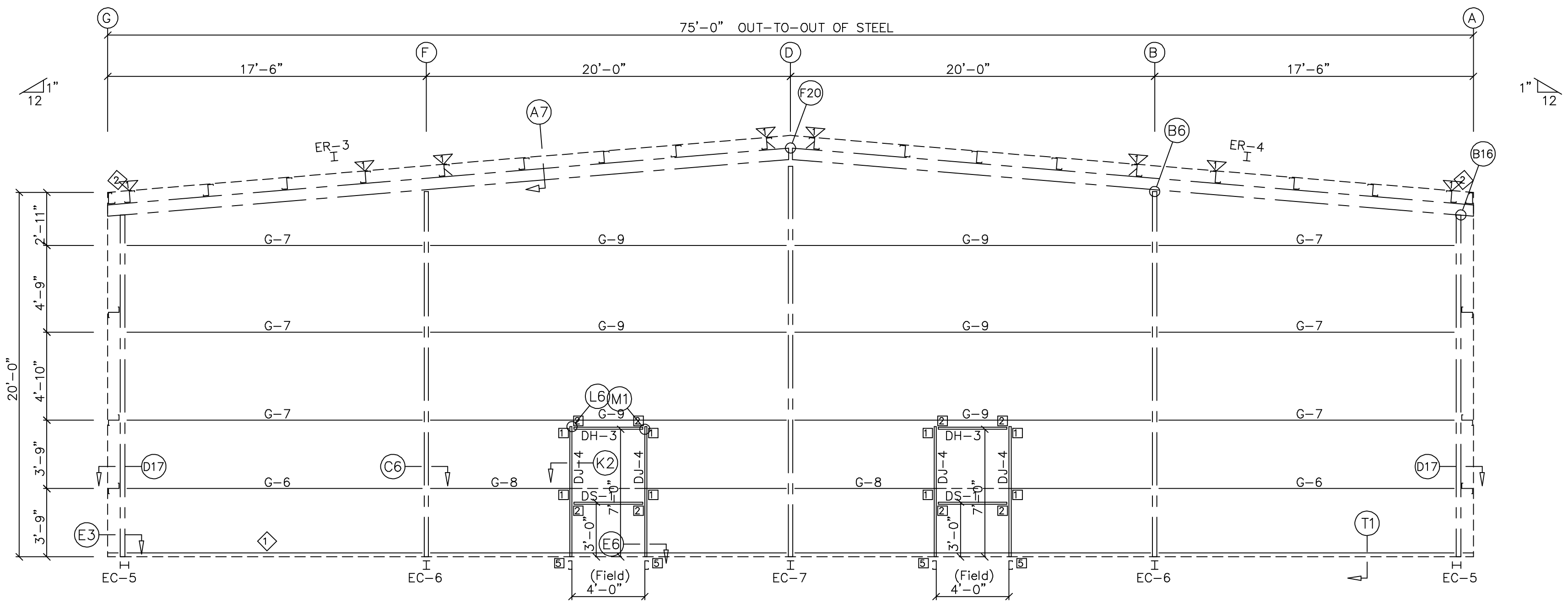
ENDWALL ELEVATION

Revision	Date	Description	By	Ckk'd

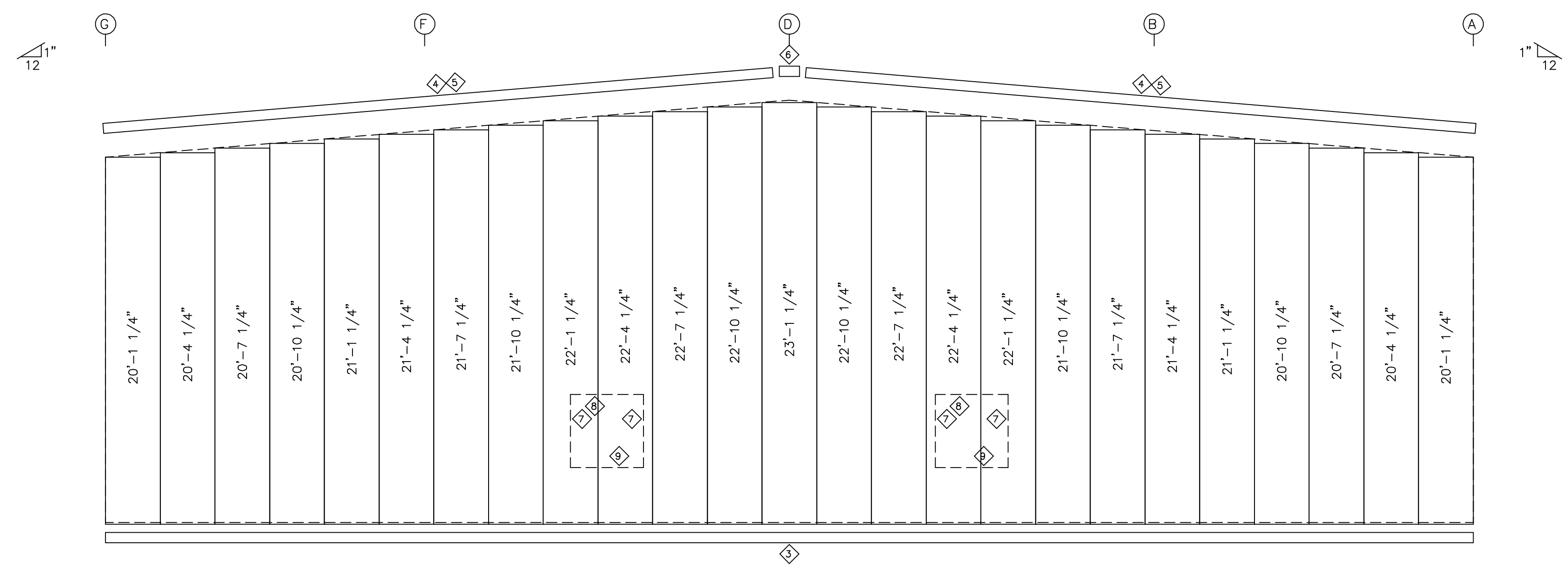
Drawing Description: ENDWALL ELEVATION
Customer Name: American Steel Buildings
Project Name: Ramirez Business Park - B
Project Location: Cypress, TX 77429
Job No: Q22022
Sheet No:

Panel Table
Frame Line - 6

Quantity	Length	Part	Color
2	20'-1 1/4"	26 Ga. Rloc	Gallery SMP Life
2	20'-4 1/4"	26 Ga. Rloc	Gallery SMP Life
2	20'-7 1/4"	26 Ga. Rloc	Gallery SMP Life
2	20'-10 1/4"	26 Ga. Rloc	Gallery SMP Life
2	21'-1 1/4"	26 Ga. Rloc	Gallery SMP Life
2	21'-4 1/4"	26 Ga. Rloc	Gallery SMP Life
2	21'-7 1/4"	26 Ga. Rloc	Gallery SMP Life
2	21'-10 1/4"	26 Ga. Rloc	Gallery SMP Life
2	22'-1 1/4"	26 Ga. Rloc	Gallery SMP Life
2	22'-4 1/4"	26 Ga. Rloc	Gallery SMP Life
2	22'-7 1/4"	26 Ga. Rloc	Gallery SMP Life
2	22'-10 1/4"	26 Ga. Rloc	Gallery SMP Life
1	23'-1 1/4"	26 Ga. Rloc	Gallery SMP Life



ENDWALL FRAMING: Frame Line 6



ENDWALL SHEETING & TRIM: Frame Line 6

American Steel Buildings
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Revision	Date	Description	By	Chk'd

Drawing Description: ENDWALL ELEVATION

Customer Name: American Steel Buildings

Project Name: Ramirez Business Park – B

Project Location: Cypress, TX 77429

Job No: Q22022

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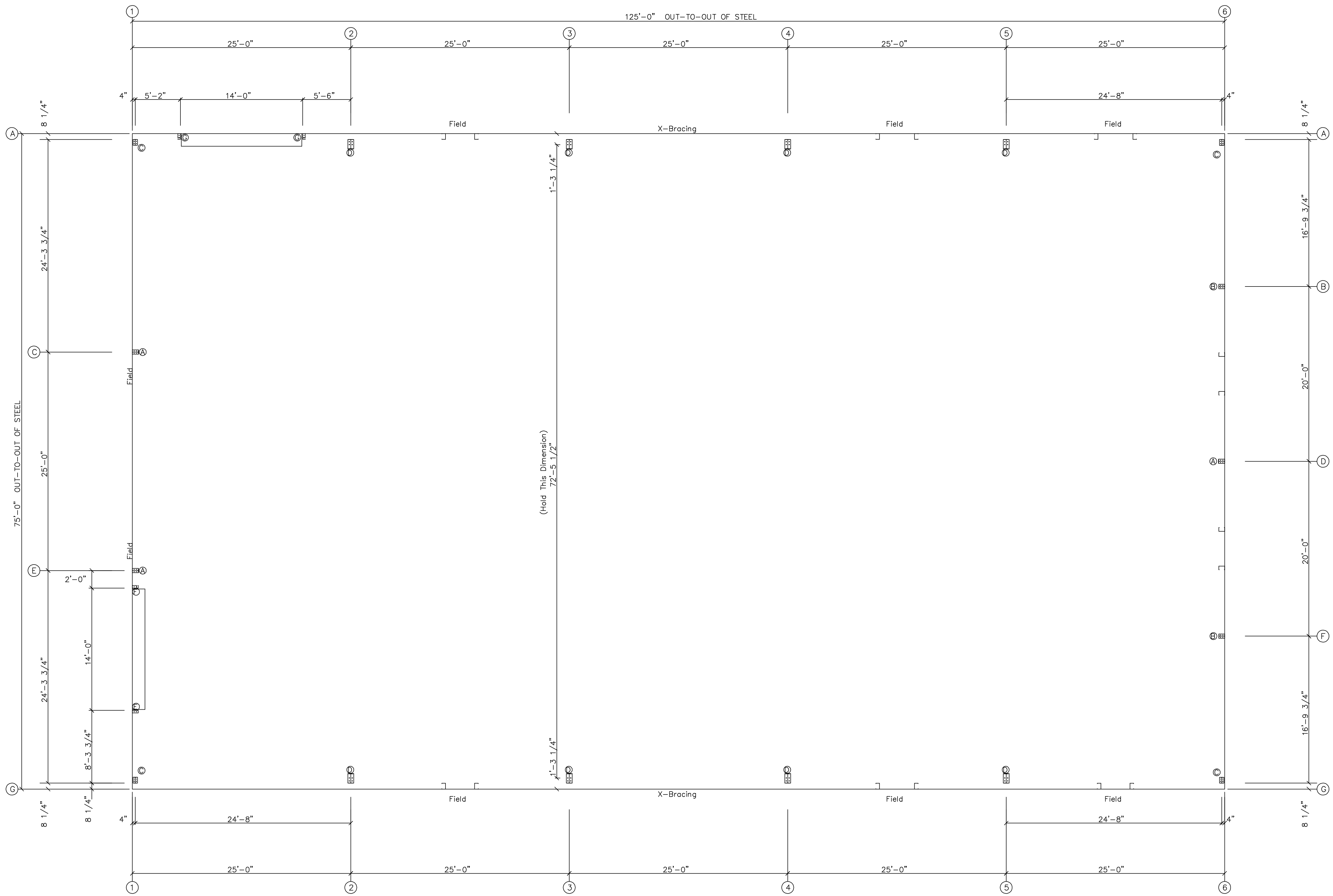

ANCHOR BOLT PLAN

NOTE: All Base Plates @ 100'-0" (U.N.)

GENERAL NOTES

1.) Anchor Rods are not furnished or installed by the manufacturer.
2.) No Grout is to be used under the base plates unless noted otherwise.
3.) The Metal Building Manufacturer is not responsible for the design, materials, or Workmanship of the foundation. Anchor Rod Plans prepared by the manufacturer are intended to show only location, diameter, and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end Customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 06 Sections 3.2.2 and A3)
4.) Foundation must be square and level with all anchor rods true in size, location, and projection.

American Steel Buildings
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Drawing Description: ANCHOR BOLT PLAN

Customer Name: American Steel Buildings

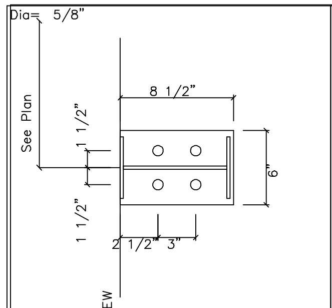
Project Name: Ramirez Business Park - B

Project Location: Cypress, TX 77429

Job No: Q22022

Sheet No:

Revision	Date	Description	By	Chk'd



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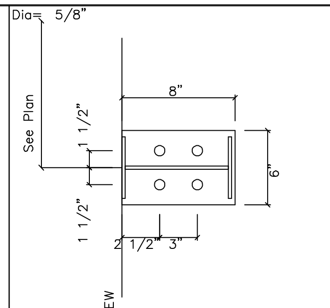

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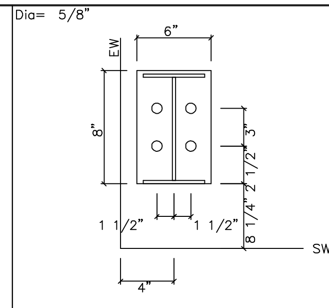

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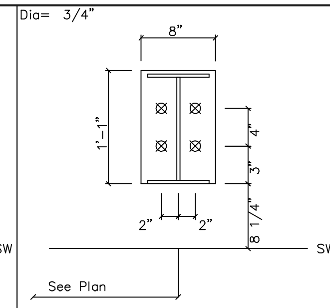

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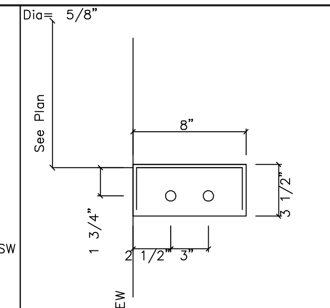

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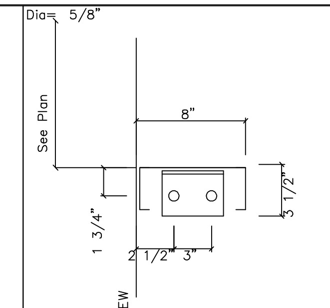

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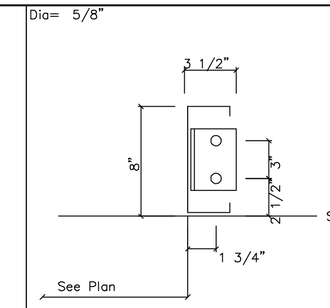

DETAIL G

American Steel Buildings
PO Box 12327
Murfreesboro, TN. 37129
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Drawing Description: "A"

Customer Name: American Steel Buildings

Project Name: Ramirez Business Park - B

Project Location: Cypress, TX 77429

Job No: Q22022

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Revision	Date	Description	By	Chk'd

NOTES FOR REACTIONS

1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Building reactions are based on the following building data:

Width	(ft)	=	75.0
Length	(ft)	=	125.0
Eave Height	(ft)	=	20.0 / 20.0
Roof Slope	(rise/12)	=	1.00 / 1.00
Roof Dead Load	(psf)	=	2.5
Wall Dead Load			
Left Endwall	(psf)	=	2.0
Right Endwall	(psf)	=	2.0
Front Sidewall	(psf)	=	2.0
Back Sidewall	(psf)	=	2.0
Roof Live Load	(psf)	=	20.0
Frame Live Load	(psf)	=	12.0
Collateral Load	(psf)	=	3.0
Snow Load	(psf)	=	3.5
Minimum Snow	(psf)	=	5.0
Wind Speed	(mph)	=	130.0
Wind Code		=	IBC 21
Exposure		=	C
Closure		=	Enclosed
Internal Wind Coeff		=	-0.18, +0.18
Risk Category		=	II -- Normal
Importance -- Wind		=	1.00
Importance -- Seismic		=	1.00
Seismic Design Category		=	A
Seismic Coeff	(Sms)	=	0.11

5. Loading conditions are:

1 Dead+Collateral+Live
2 0.6Dead+0.6Wind_Left1
3 0.6Dead+0.6Wind_Right1
4 0.6Dead+0.6Wind_Long1L
5 0.6Dead+0.6Wind_Long2L
6 0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
7 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
8 Dead+Collateral+E1PAT_LL_3
9 0.6Dead+0.6Wind_Left1+0.6Wind_Suction
10 Dead+Collateral+E1PAT_LL_1
11 0.6Dead+0.6Wind_Right1+0.6Wind_Suction
12 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
13 Dead+Collateral+E1PAT_LL_2
14 0.6Dead+0.6Wind_Suction+0.6Wind_Long2L
15 Dead+Collateral+E2PAT_LL_4
16 Dead+Collateral+E2PAT_LL_1
17 Dead+Collateral+E2PAT_LL_2
18 Dead+Collateral+E2PAT_LL_3
19 Dead+Collateral+E2PAT_LL_5

GENERAL NOTES

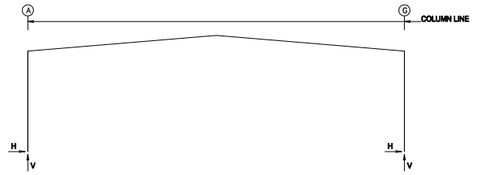
It is the repsonsibility of the end user to verify that the loads shown meet local requirements and are adequate for the intended use of the building. Metal building manufacturer does not serve as the Engineer of Record.

Metal building manufacturer is not responsible for the fit of framing steel in instances where anchorbolts are not set in the exact locations shown on these drawing.

The horizontal load from the building bracing reactions are perpendicular to the Rigid frame horizontal reactions. The building bracing reactions are to be combined with the loads from the rigid frame reactions.

The anchor bolts are ASTM F1554 Gr. 36. The anchor bolt projection starts at bottom of base plate. The base plate design is based on minimum 3000 psi concrete compressive strength.

FRAME LINES: 2 3 4 5



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Column_Reactions(k) Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Grout (in)
2*	A	1	12.8	19.6	2	-10.1	-13.3	4	0.750	8.000	13.00	0.625	0.0
					4	-7.2	-22.7						
2*	G	3	10.1	-13.3	1	-12.8	19.6	4	0.750	8.000	13.00	0.625	0.0
		1	-12.8	19.6	5	7.2	-22.7						

2* **Frame lines:** 2 3 4 5

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horz	Dead Vert	Collateral Horz	Collateral Vert	Live Horz	Live Vert	Snow Horz	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert
2*	A	2.1	3.6	3.2	4.7	7.6	11.3	2.2	3.3	-18.9	-25.8
2*	G	-2.1	3.6	-3.2	4.7	-7.6	11.2	-2.2	3.3	8.3	-18.0

Frame Line	Column Line	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert
2*	A	-8.3	-18.0	-14.4	-14.7	-3.9	-8.9	-14.0	-41.5	-15.4	-33.7
2*	G	18.9	-25.8	3.9	-6.9	14.4	-14.7	15.4	-33.7	14.0	-41.5

Frame Line	Column Line	Seismic_Left Horz	Seismic_Left Vert	Seismic_Right Horz	Seismic_Right Vert	Seismic_Long1 Horz	Seismic_Long1 Vert	Seismic_Long2 Horz	Seismic_Long2 Vert	-MIN_SNOW- Horz	-MIN_SNOW- Vert
2*	A	-0.2	-0.1	0.2	0.1	0.0	-0.9	0.0	0.9	3.1	4.7
2*	G	-0.2	0.1	0.2	-0.1	0.0	-0.9	0.0	0.9	-3.1	4.7

Frame Line	Column Line	E1UNB_SL_L- Horz	E1UNB_SL_L- Vert	F1UNB_SL_R- Horz	F1UNB_SL_R- Vert
2*	A	2.3	3.7	2.3	2.3
2*	G	-2.3	2.3	-2.3	3.7

2* **Frame lines:** 2 3 4 5

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horz	Wind Suct Horz	Wind Long1 Vert	Wind Long2 Vert
1	A	0.7	0.7	2.3	0.5	-5.0	-2.8	-3.3	-1.1	-3.4	3.9	-5.4	-3.2
1	C	1.7	1.7	6.1	1.2	-13.4	-8.0	-8.4	-4.1	-7.3	8.1	-13.0	-7.6
1	E	1.7	1.7	6.1	1.2	-8.0	-13.4	-4.1	-9.4	-7.3	8.1	-7.6	-13.0
1	G	0.7	0.7	2.3	0.5	-2.8	-5.0	-1.1	-3.3	-3.4	3.9	-3.2	-5.4

Frm Line	Col Line	Sole Left Vert	Sole Right Vert	Sole Long Vert	-MIN_SNOW- Horz	-MIN_SNOW- Vert	E1UNB_SL_L- Horz	E1UNB_SL_L- Vert	E1UNB_SL_R- Horz	E1UNB_SL_R- Vert	E1PAT_LL_1- Horz	E1PAT_LL_1- Vert	E1PAT_LL_2- Horz	E1PAT_LL_2- Vert
1	A	0.0	0.0	0.0	0.0	0.7	0.0	0.4	0.0	0.1	0.0	2.2	0.0	-0.2
1	C	0.0	0.0	0.0	0.0	1.7	0.0	1.9	0.0	0.6	0.0	6.6	0.0	2.6
1	E	0.0	0.0	0.0	0.0	1.7	0.0	0.6	0.0	1.9	0.0	2.6	0.0	6.6
1	G	0.0	0.0	0.0	0.0	0.7	0.0	0.1	0.0	0.4	0.0	-0.2	0.0	2.2

Frm Line	Col Line	E1PAT_LL_3- Horz	E1PAT_LL_3- Vert	E1PAT_LL_4- Horz	E1PAT_LL_4- Vert
1	A	0.0	2.6	0.0	-0.3
1	C	0.0	3.0	0.0	3.1
1	E	0.0	3.0	0.0	3.1
1	G	0.0	2.6	0.0	-0.3

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horz	Wind Suct Horz	Wind Long1 Vert	Wind Long2 Vert
6	G	0.5	0.4	1.7	0.3	-3.3	-2.0	-2.1	-0.8	-3.4	3.9	-3.6	-2.3
6	F	1.1	1.3	5.1	0.9	-10.5	-5.6	-7.4	-2.5	-5.3	5.9	-10.5	-5.6
6	D	1.2	1.3	5.0	0.9	-7.8	-7.8	-5.1	-5.1	-6.3	6.9	-7.2	-7.2
6	B	1.1	1.3	5.1	0.9	-5.6	-10.5	-2.5	-7.4	-5.3	5.9	-5.6	-10.5
6	A	0.5	0.4	1.7	0.3	-2.0	-3.3	-0.8	-2.1	-3.4	3.9	-2.3	-3.6

Frm Line	Col Line	Sole Left Vert	Sole Right Vert	Sole Long Vert	-MIN_SNOW- Horz	-MIN_SNOW- Vert	E2UNB_SL_L- Horz	E2UNB_SL_L- Vert	E2UNB_SL_R- Horz	E2UNB_SL_R- Vert	E2PAT_LL_1- Horz	E2PAT_LL_1- Vert	E2PAT_LL_2- Horz	E2PAT_LL_2- Vert
6	G	0.0	0.0	0.0	0.0	0.4	0.0	0.3	0.0	0.1	0.0	1.7	0.0	-0.2
6	F	0.0	0.0	0.0	0.0	1.3	0.0	1.3	0.0	0.1	0.0	5.4	0.0	2.4
6	D	0.0	0.0	0.0	0.0	1.3	0.0	1.3	0.0	1.3	0.0	2.5	0.0	5.7
6	B	0.0	0.0	0.0	0.0	1.3	0.0	0.1	0.0	1.3	0.0	-0.3	0.0	2.4
6	A	0.0	0.0	0.0	0.0	0.4	0.0	0.1	0.0	0.3	0.0	0.1	0.0	-0.2

Frm Line	Col Line	E2PAT_LL_3- Horz	E2PAT_LL_3- Vert	E2PAT_LL_4- Horz	E2PAT_LL_4- Vert	E2PAT_LL_5- Horz	E2PAT_LL_5- Vert
6	G	0.0	0.1	0.0	2.1	0.0	-0.3
6	F	0.0	-0.3	0.0	2.2	0.0	2.9
6	D	0.0	2.5	0.0	2.5	0.0	2.5
6	B	0.0	5.4	0.0	2.9	0.0	2.2
6	A	0.0	1.7	0.0	-0.3	0.0	2.1

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Column_Reactions(k) Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in) Width	Length	Thick	Grout (in)
1	A	6	2.3	-2.8	7	-2.0	-2.8	4	0.625	6.000	8.000	0.375	0.0
		8	0.0	4.0	6	2.3	-2.8						
1	C	9	4.9	-7.0	7	-4.4	-6.8	4	0.625	6.000	8.500	0.375	0.0
		10	0.0	10.0	9	4.9	-7.0						
1	E	11	4.9	-7.0	12	-4.4	-6.8	4	0.625	6.000	8.500	0.375	0.0
		13	0.0	10.0	11	4.9	-7.0						
1	G	14	2.3	-2.8	12	-2.0	-2.8	4	0.625	6.000	8.000	0.375	0.0
		8	0.0	4.0	14	2.3	-2.8						
6	G	6	2.3	-1.9	7	-2.0	-1.9	4	0.625	6.000	8.000	0.375	0.0
		15	0.0	3.0	6	2.3	-1.9						
6	F	9	3.5	-5.7	7	-3.2	-5.7	4	0.625	6.000	8.000	0.375	0.0
		16	0.0	7.8	9	3.5	-5.7						
6	D	11	4.1	-4.0	12	-3.8	-3.6	4	0.625	6.000	8.500	0.375	0.0
		17	0.0	8.1	11	4.1	-4.0						
6	B	11	3.5	-5.7	12	-3.2	-5.7	4	0.625	6.000	8.000	0.375	0.0
		18	0.0	7.8	11	3.5	-5.7						
6	A	14	2.3	-1.9	12	-2.0	-1.9	4	0.625	6.000	8.000	0.375	0.0
		19	0.0	3.0	14	2.3	-1.9						

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
○ 8	Jamb	5/8"		
○ 36	Endwall	5/8"	A307	2.50
⊠ 32	Frame	3/4"	A307	3.00

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	↑ Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Seis
L_EW	1						57	4
F_SW	C	3,4	10.1	7.4	1.2	0.9		
R_EW	6						52	4
B_SW	A	4,3	10.1	7.4	1.2	0.9		

Reactions for seismic represent shear force, Eh
Reaction values shown are unfactored

PO Box 12327
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American Steel Buildings

ANCHOR BOLT REACTIONS

Drawing Description:	Customer Name:	Project Name:	Project Location:	Job No:	Sheet No:
	American Steel Buildings	Ramirez Business Park - B	Cypress, TX. 77429	Q22022	

Revision	Date	Description	By	Ck'd