

Wheeler Metals, Inc.



3100 West 40th. Street North
Muskogee, Oklahoma 74401

BUILDING LOADS / DESCRIPTION:

WIDTH: 50 LENGTH: 170 HEIGHT: 13 /17.17
(BUILDING DIMENSIONS ARE NOMINAL. REFER TO PLANS).

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY : IBC 15 / AISC10 / NAUS12 .

THE CONTRACTOR IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

ROOF DEAD LOAD: 2,000 PSF (ROOF PANELS & PURLINS)

COLLATERAL LOAD: 0 PSF

ROOF LIVE LOAD: PRIMARY: 20.00 PSF
SECONDARY: 20 PSF

GROUND SNOW LOAD: 15 PSF

ROOF SNOW LOAD: 10.5 PSF

DESIGN WIND SPEED: 115 MPH

SEISMIC ZONE: C

WIND EXPOSURE: C

IMPORTANCE FACTORS:
WIND LOAD: 1.00
SNOW LOAD 1.0000
SEISMIC LOAD 1.00

GENERAL NOTES:
1) MATERIALS :
HOT ROLLED BAR Fy = 50,0000 ksi MIN.
STRUCTURAL STEEL SHEET Fy = 50,0000 ksi MIN.
STRUCTURAL STEEL PLATE Fy = 50,0000 ksi MIN.
COLD FORMED SHAPES Fy = 57,0000 ksi MIN.
WALL SHEETING Fy = 80,0000 ksi MIN.
ROOF SHEETING Fy = 80,0000 ksi MIN.
BOLTS A307 & A325
THE METAL BUILDING MANUFACTURER RESERVES THE RIGHT TO SUBSTITUTE THE ABOVE MATERIALS WITH EQUAL OR BETTER MATERIAL.

2) BOLT TIGHTENING REQUIREMENTS:
ALL HIGH STRENGTH BOLTS ARE A325 UNLESS NOTED OTHERWISE. HIGH STRENGTH BOLTS SHALL BE TIGHTENED BY THE TURN OF THE NUT METHOD IN ACCORDANCE WITH THE LATEST EDITION AISC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". A325 BOLTS SHALL BE INSTALLED WITH OUT WASHERS WHEN TIGHTENED BY THE "TURN OF THE NUT" METHOD. ALL BOLTED CONNECTIONS, FOR SHEAR/BEARING CONNECTION TYPE WITH BOLT THREADS EXCLUDED FROM THE SHEAR PLANE SHALL BE SNGO TIGHT ONLY.

3) ALL STRUCTURAL STEEL TO RECEIVE A RUST INHIBITIVE PRIMER. THIS PAINT IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS.

ROOF PANELS:

COLOR: Ash Gray

WALL PANELS:

COLOR: Burnished Slate

TRIM COLORS:

GABLE: Burnished Slate

CORNER: Burnished Slate

EAVE: Burnished Slate

FRAMED OPENINGS: Burnished Slate

DEFLECTION LIMITS:

EW COL: 90
EW RAF LIVE: 180
EW RAF WIND: 150
WALL GIRT: 90
PURL LIVE: 150
PURL WIND: 120
WALL PANEL: 90
ROOF PANEL LIVE: 150
ROOF PANEL WIND: 120
RF HORIZONTAL: 60
RF VERTICAL: 180
WIND BENT: 60
RF CRANE: 100
RF SEIS: 50
WIND BENT SEIS: 50

BUILDER / CONTRACTOR RESPONSIBILITIES

IT IS THE RESPONSIBILITY OF THE BUILDER/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT THE METAL BUILDING SYSTEM MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.

APPROVAL OF THE METAL BUILDING SYSTEM MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE METAL BUILDING SYSTEM MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.2.1 AISC CODE OF STANDARD PRACTICES, 9TH ED.)

WHERE DISCREPANCIES EXIST BETWEEN THE METAL BUILDING SYSTEM MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE, 9TH ED.)

DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE METAL BUILDING SYSTEM MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE METAL BUILDING SYSTEM MANUFACTURER'S ENGINEER UNLESS SPECIFICALLY INDICATED.

THE CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH THE METAL BUILDING SYSTEM MANUFACTURER "FOR CONSTRUCTION" DRAWINGS.

ALL BRACING AS SHOWN AND PROVIDED BY THE METAL BUILDING SYSTEM MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.

TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION, OR COLLISION. (SECT. 7.9.1 AISC CODE OF STANDARD PRACTICE, 9TH ED.)

WARNING: IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSION EFFECTS ON THE ALUMINUM ZINC ALLOY COATING WHEN THEY ARE USED IN CONTACT WITH GALVALUME STEEL PANELS. EVEN RUN-OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO GALVALUME SHOULD BE AVOIDED.

APPROVAL NOTES

THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS: IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS BE MADE IN CONTRASTING INK (PREFERABLY RED INK), HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED, AND BE LEGIBLE AND UNAMBIGUOUS.

A SIGNATURE AND DATE IS REQUIRED ON ALL PAGES.

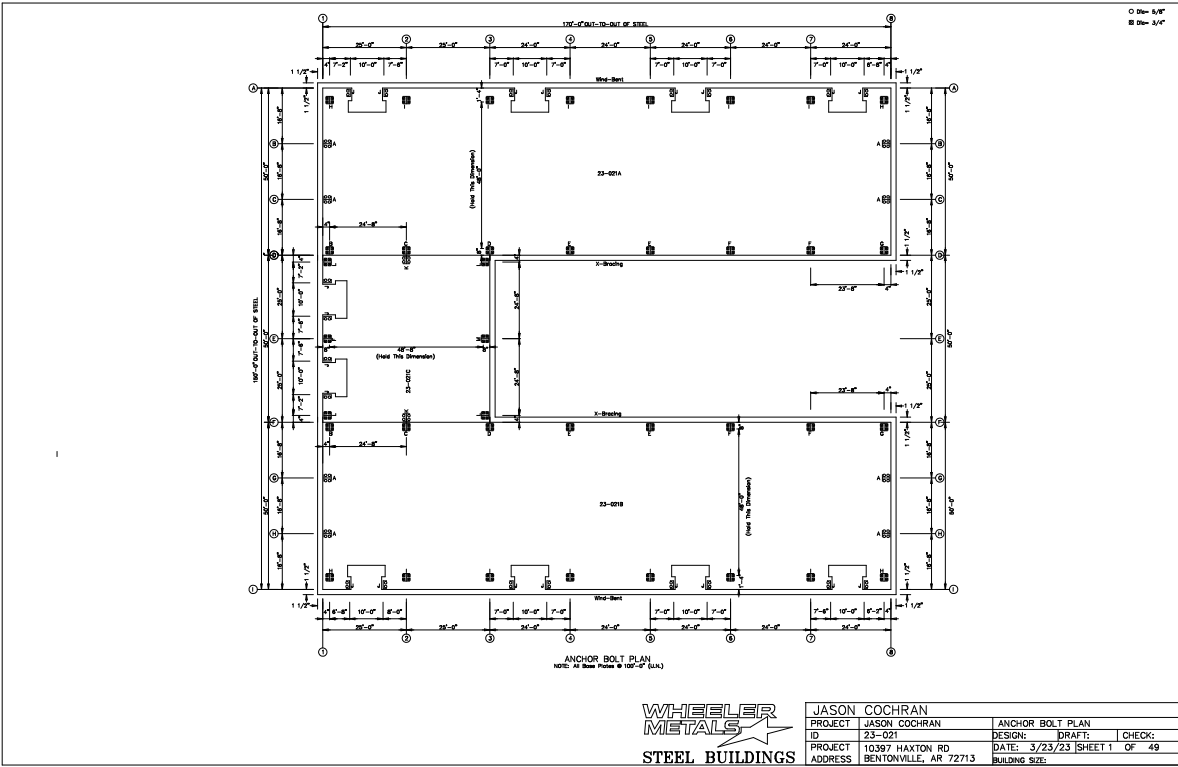
MANUFACTURER RESERVES THE RIGHT TO RE-SUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE DELIVERY SCHEDULE.

APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE METAL BUILDING SYSTEM MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED BY MANUFACTURER. ANY CHANGES NOTED ON THE DRAWINGS NOT IN CONFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION. MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILAR INDICATION OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS UNACCEPTABLE ALTERNATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER.

IMPORTANT NOTE: FINAL DETAILING, FABRICATION, AND DELIVERY DATE OF THIS PROJECT CANNOT BE COMPLETED UNTIL THE SIGNED APPROVALS ARE RETURNED TO THE METAL BUILDING MANUFACTURER.

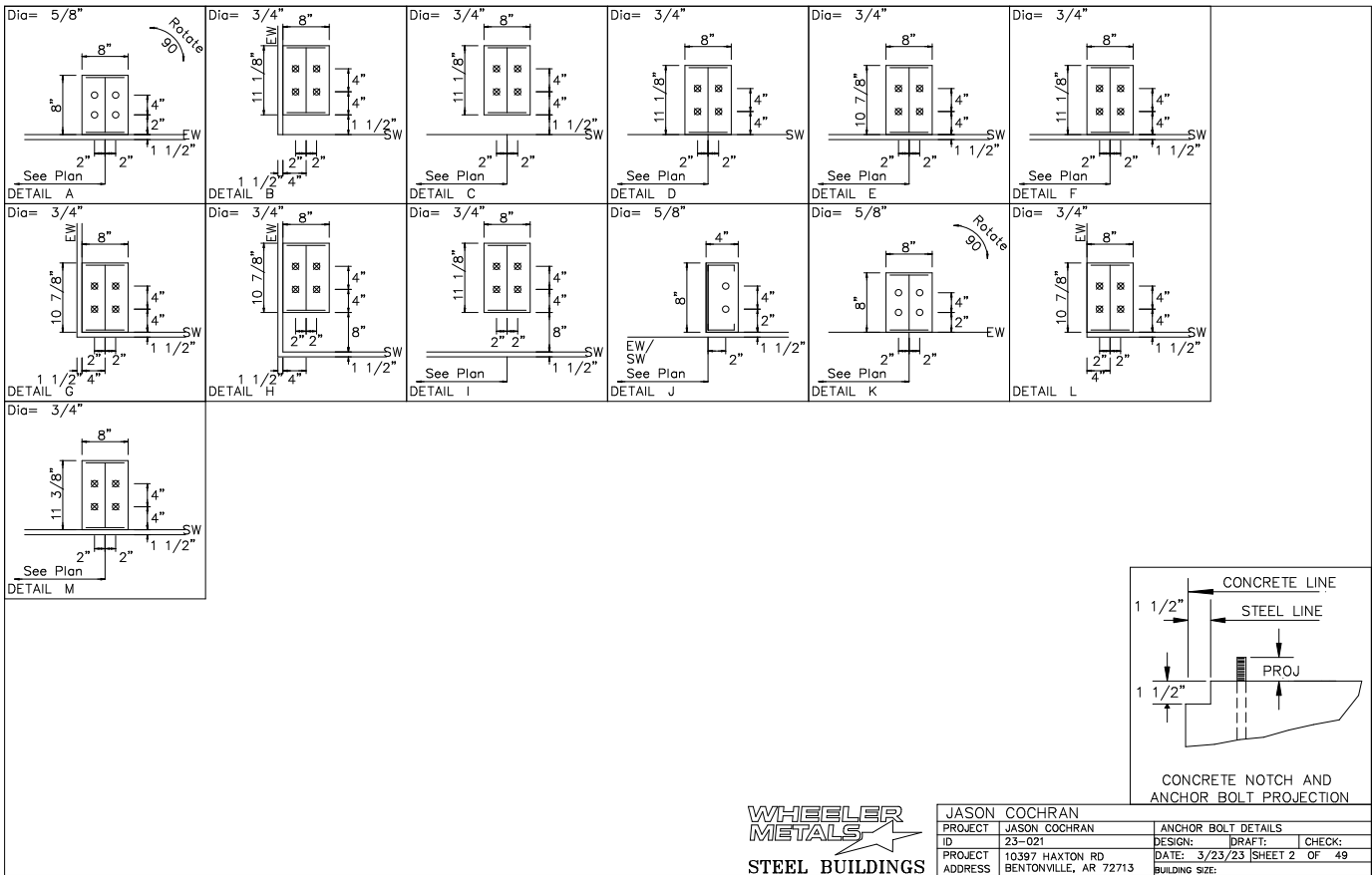
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△	FOR CONSTRUCTION	
△	FOR APPROVAL	
REV	DATE	REVISION

JOB NUMBER: 23-021A
PURCHASER: JASON COCHRAN
PROJECT: JASON COCHRAN
ADDRESS: 10397 HAXTON RD
BENTONVILLE, AR



WHEELER
METALS
 STEEL BUILDINGS

PROJECT JASON COCHRAN		ANCHOR BOLT PLAN	
ID 23-021	DESIGN:	DRAFT:	CHECK:
PROJECT 10397 HAXTON RD	DATE: 3/23/23	SHEET 1 OF 49	
ADDRESS BENTONVILLE, AR 72713	BUILDING SIZE:		



WHEELER METALS
 STEEL BUILDINGS

JASON COCHRAN		
PROJECT	JASON COCHRAN	ANCHOR BOLT DETAILS
ID	23-021	DESIGN: DRAFT: CHECK:
PROJECT ADDRESS	10397 HAXTON RD BENTONVILLE, AR 72713	DATE: 3/23/23 SHEET 2 OF 49 BUILDING SIZE:

NOTES FOR REACTIONS

Building reactions are based on the following loading data:

- Wind (F) = 50.0
- Length (ft) = 170.0
- Eave height (ft) = 13.0/ 17.2
- Roof Slope (W/12) = 0.0
- Dead Load (psf) = 2.0
- Collators (psf) = 0.0
- Live Load (psf) = 20.0
- Snow Load (psf) = 15.0
- Wind Speed (mph) = 115.0
- Exposure = C
- Close/Open = C
- Importance = 1.00
- Importance Seismic = 1.00
- Seismic Zone = C
- Seismic Coeff (F/Sa) = 0.25

ID Description

- 1 Dead+Collateral+Live
- 2 0.0Dead+0.0Wind+Left
- 3 0.0Dead+0.0Wind+Right
- 4 0.0Dead+0.0Wind+Long1
- 5 0.0Dead+0.0Wind+Long2
- 6 0.0Dead+0.0Wind+Pressure+0.0Wind+Long1
- 7 0.0Dead+0.0Wind+Pressure+0.0Wind+Long2
- 8 0.0Dead+0.0Wind+Pressure+0.0Wind+Section

BUILDING BRACING REACTIONS

Loc	Line	Wind	Seismic	Panel	Shear
Line	Line	Horz	Vert	Wind	Sels
L-W	1	4.5	3.7	2.4	0.7
R-W	8	4.5	1.7	1.6	0.5

(n) Wind bent in bay base above finish floor
(o) Rigid frame at endwall

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame	Column	Line	Dead	Live	Snow	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2
1*	A	1	-0.5	1.2	-2.9	8.4	-1.5	3.4	-2.2
1*	D	1	-0.5	1.2	-2.9	8.4	-1.5	3.4	-2.2

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame	Col	Line	Load	Reactions (k)	Bolt (k)	Base Plate (in)	Grout (in)
Line	Line	Line	Id	Horz	Vert	Qty	Dia
1*	A	1	1	-3.3	7.6	4	0.750
1*	D	1	1	-3.3	7.6	4	0.750

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

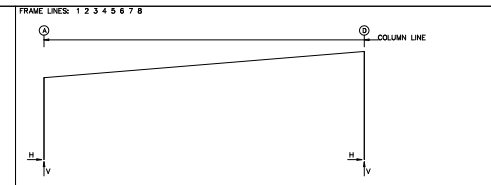
Frame	Col	Line	Dead	Live	Wind	Seismic
1	B	1	0.2	2.3	2.8	0.2
1	B	2	0.2	2.3	2.8	0.2

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame	Col	Line	Load	Reactions (k)	Bolt (k)	Base Plate (in)	Grout (in)
Line	Line	Line	Id	Horz	Vert	Qty	Dia
1	B	1	1	1.5	0.1	4	0.625
1	C	1	1	1.7	0.1	4	0.625

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Load (k)	Embed (in)	Prod (in)
0 18	Jamb	5/8"	A307	10.0	3.00	3.00
0 18	Endwall	5/8"	A307	10.0	3.00	3.00
0 64	Frame	5/8"	A307	18.0	3.00	3.00



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame	Col	Line	Load	Reactions (k)	Bolt (k)	Base Plate (in)	Grout (in)
Line	Line	Line	Id	Horz	Vert	Qty	Dia
1*	A	1	1	-3.3	7.6	4	0.750
1*	D	1	1	-3.3	7.6	4	0.750

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame	Col	Line	Load	Reactions (k)	Bolt (k)	Base Plate (in)	Grout (in)
Line	Line	Line	Id	Horz	Vert	Qty	Dia
2*	A	1	1	8.6	14.5	4	0.750
2*	D	1	1	-8.9	14.4	4	0.750

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame	Col	Line	Load	Reactions (k)	Bolt (k)	Base Plate (in)	Grout (in)
Line	Line	Line	Id	Horz	Vert	Qty	Dia
3*	A	1	1	-8.3	13.9	4	0.750
3*	D	1	1	-8.3	13.9	4	0.750

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame	Col	Line	Load	Reactions (k)	Bolt (k)	Base Plate (in)	Grout (in)
Line	Line	Line	Id	Horz	Vert	Qty	Dia
4*	A	1	1	-8.5	14.0	4	0.750
4*	D	1	1	-8.3	13.9	4	0.750

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame	Col	Line	Load	Reactions (k)	Bolt (k)	Base Plate (in)	Grout (in)
Line	Line	Line	Id	Horz	Vert	Qty	Dia
5*	A	1	1	-4.0	7.5	4	0.750
5*	D	1	1	-4.0	7.5	4	0.750



JASON COCHRAN			
PROJECT	JASON COCHRAN	ANCHOR BOLT REACTIONS	
ID	23-021	DESIGN	DRAFT
PROJECT	10397 HAXTON RD	DATE	3/23/23
ADDRESS	BENTONVILLE, AR 72713	SHEET	3 OF 49
BUILDING SIZE:			

NOTES FOR REACTIONS

Building reactions are based on the following loading data:

- Wind (F) = 50.0
- Length (ft) = 170.0
- Eave height (ft) = 13.0/ 17.2
- Roof Slope (W/12) = 1.0
- Dead Load (psf) = 2.0
- Collators (psf) = 0.0
- Live Load (psf) = 20.0
- Snow Load (psf) = 15.0
- Wind Speed (mph) = 115.0
- Exposure = C
- Importance Wind = 1.00
- Importance Seismic = 1.00
- Seismic Zone = C
- Seismic Coeff (F/Sa) = 0.25

ID Description

- 1 Dead+Collateral+Live
- 2 0.0Dead+0.0Wind+Left
- 3 0.0Dead+0.0Wind+Right
- 4 0.0Dead+0.0Wind+Long1
- 5 0.0Dead+0.0Wind+Long2
- 6 0.0Dead+0.0Wind+Pressure+0.0Wind+Long2
- 7 0.0Dead+0.0Wind+Pressure+0.0Wind+Long2
- 8 0.0Dead+0.0Wind+Pressure+0.0Wind+Long2

BUILDING BRACING REACTIONS

Loc	Line	Wind	Seismic	Panel	Shear	
Line	Line	Horz	Vert	Wind	Seis	
L-W	B	5.4	3.7	2.4	0.7	0.4
R-W	I	5.4	1.7	1.6	0.5	0.5

(n) Wind bent in bay base above finish floor
(o) Rigid frame at eave

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame	Column	Line	Dead	Live	Snow	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2
8*	I	1	0.6	1.3	3.6	8.4	1.8	3.4	8.2
8*	F	1	-0.6	-1.3	-3.6	-8.4	-1.8	-3.4	-8.2

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

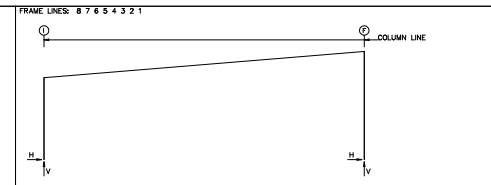
Frame	Col	Line	Dead	Press	Wind	Suct
8*	H	1	0.2	2.3	2.8	2.8
8*	G	1	0.2	2.3	2.8	2.8
8*	I	1	0.2	2.3	2.8	2.8

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame	Col	Line	Load	Horz	Vert	Bolt(n)	Base_Plate(n)	Grout
Line	Line	Line	Line	Line	Line	Qty	Width	Length
Line	Line	Line	Line	Line	Line	Qty	Width	Length
B	H	1	6	1.5	0.1	4	0.625	8.000
B	G	1	6	1.7	0.1	4	0.625	8.000
1	G	1	6	1.7	0.1	4	0.625	8.000
1	H	1	6	1.5	0.1	4	0.625	8.000

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Lead (in)	Band (in)	Prod (in)
0 18	Jamb	5/8"	A307	10.0	3.00	3.00
0 18	Endwall	5/8"	A307	10.0	3.00	3.00
0 64	Frame	5/8"	A307	18.0		



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame	Col	Line	Load	Horz	Vert	Bolt(n)	Base_Plate(n)	Grout
Line	Line	Line	Line	Line	Line	Qty	Width	Length
Line	Line	Line	Line	Line	Line	Qty	Width	Length
8*	I	1	4.2	7.6	0.0	4	0.750	8.000
8*	F	1	-4.2	-7.6	0.0	4	0.750	8.000

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame	Col	Line	Load	Horz	Vert	Bolt(n)	Base_Plate(n)	Grout
Line	Line	Line	Line	Line	Line	Qty	Width	Length
Line	Line	Line	Line	Line	Line	Qty	Width	Length
7*	I	1	8.6	14.5	0.0	4	0.750	8.000
7*	F	1	-8.6	-14.5	0.0	4	0.750	8.000

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame	Col	Line	Load	Horz	Vert	Bolt(n)	Base_Plate(n)	Grout
Line	Line	Line	Line	Line	Line	Qty	Width	Length
Line	Line	Line	Line	Line	Line	Qty	Width	Length
8*	I	1	8.3	14.5	0.0	4	0.750	8.000
8*	F	1	-8.3	-14.5	0.0	4	0.750	8.000

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame	Col	Line	Load	Horz	Vert	Bolt(n)	Base_Plate(n)	Grout
Line	Line	Line	Line	Line	Line	Qty	Width	Length
Line	Line	Line	Line	Line	Line	Qty	Width	Length
9*	I	1	8.5	14.5	0.0	4	0.750	8.000
9*	F	1	-8.5	-14.5	0.0	4	0.750	8.000

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frame	Col	Line	Load	Horz	Vert	Bolt(n)	Base_Plate(n)	Grout
Line	Line	Line	Line	Line	Line	Qty	Width	Length
Line	Line	Line	Line	Line	Line	Qty	Width	Length
10*	I	1	3.3	7.6	0.0	4	0.750	8.000
10*	F	1	-3.3	-7.6	0.0	4	0.750	8.000



JASON COCHRAN			
PROJECT	JASON COCHRAN	ANCHOR BOLT REACTIONS	
ID	23-021	DESIGN	DRAFT
PROJECT	10397 HAXTON RD	DATE:	3/23/23
ADDRESS	BENTONVILLE, AR 72713	SHEET	4 OF 49
BUILDING SIZE:			

