

1. Any revisions to the plan must be approved by the Fire Marshal.
2. All commercial buildings must have an outside electrical disconnect.
3. Emergency egress lighting shall be provided and arranged to provide minimum candle of 1 ft candle in accordance with LSC. Emergency lighting is to be located around all exits, doors, hallways, and rooms subject to occupancy. Exit signage shall be provided in accordance to LSC. 4. Portable fire extinguishers shall be provided in accordance with NFPA 10. 5. Sprinkler riser must have a current green tag in place prior to the final inspection.

APPROVED
CITY OF WOODSTOCK
FIRE PREVENTION OFFICE
NOTE: THESE DRAWINGS WERE REVIEWED UNDER THE APPLICABLE LAWS ADOPTED AT THIS TIME EVERY EFFORT WAS MADE TO CATCH CODE VIOLATIONS, BUT THIS DOES NOT RELIEVE THE OWNER OF RESPONSIBILITY TO COMPLY WITH ITEMS MISSED OR UNKNOWN TO THE REVIEWER.

APPROVED
CITY OF WOODSTOCK
BUILDING DEPARTMENT
Note: These drawings were reviewed under applicable laws adopted at this time. Every effort was made to catch code violations, but this does not relieve the owner of responsibility to comply with items missed or unknown to the reviewer.

ANY CHANGES MADE TO THE APPROVED PLANS MUST FIRST BE APPROVED BY THE CITY OF WOODSTOCK BUILDING DEPARTMENT

South On Main Commercial Building

9236 Main Street, Suites 1C, 2C, 3C, 4C and 5C Woodstock, Georgia 30188

PROJECT DESCRIPTION: This project consists of: new, five (5) unit office/retail spaces, with prototypical "White Box" tenant spaces.
No tenant space may be occupied without permit of tenant drawings.

INDEX OF DRAWINGS

CS	Cover Sheet		
A1.1	Front & Left Elevations	S-2.3	Sections & Details
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A2.2	Roof Plan & Details	M2.0	Mechanical Plans
A3.1	Building Section & Details	M3.0	Mechanical Details & Schedules
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S-2.2	Sections & Details	E3.0	Electrical Panels & Schedules

APPLICABLE BUILDING CODES

International Building Code (IBC 2018 Edition), w/Georgia Amendments (2020)
w/ Georgia State Minimum Standard Building Code
National Electrical Code (NEC 2020 Edition) w/ Georgia State Minimum Standard Electrical Code
International Fuel Gas Code (IFGC 2018 Edition), w/Georgia Amendments (2020)
w/ Georgia State Minimum Standard Gas Code
International Mechanical Code (IMC 2018 Edition), w/Georgia Amendments (2020)
w/ Georgia State Minimum Standard Mechanical Code
International Plumbing Code (IPC 2018 Edition), w/Georgia Amendments (2020)
w/ Georgia State Minimum Standard Plumbing Code
International Energy Conservation Code (IECC 2015 Edition), w/Georgia Amendments (2020)
w/ Georgia State Minimum Standard Energy Code
International Fire Code (IFC 2018 Edition), w/Georgia Amendments (2020)
w/ Georgia State Minimum Standard Fire Prevention Code
Life Safety Code (NFPA 101 2018 Edition), w/Georgia Amendments (2020)
Americans with Disabilities Act (ADA 2010 Edition)
International Property Maintenance Code (IPMC 2018 Edition), w/Georgia Amendments (2020)
w/ Georgia State Minimum Standard Property Maintenance Code

OCCUPANCY CLASSIFICATION:
Mixed Occupancy:
Group M-Mercantile/Group B-Business
TYPE OF CONSTRUCTION:
Type V-B

TOTAL BUILDING HEIGHT:
22'-6"
STORIES:
1-
TOTAL BUILDING AREA:
7,251 Square Feet

PROJECT TEAM

Owner/Developer
South on Main LLC
J.W. Collection
2355 Log Cabin Drive
Smyrna, Georgia 30339
(828) 508-8217

Architect
Peek Design Group
3950 Cobb Parkway NW, Suite 501
Acworth, Georgia 30101
(770) 222-4345

Contractor
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2355 Log Cabin Drive
Smyrna, Georgia 30339
J.R. Merendo
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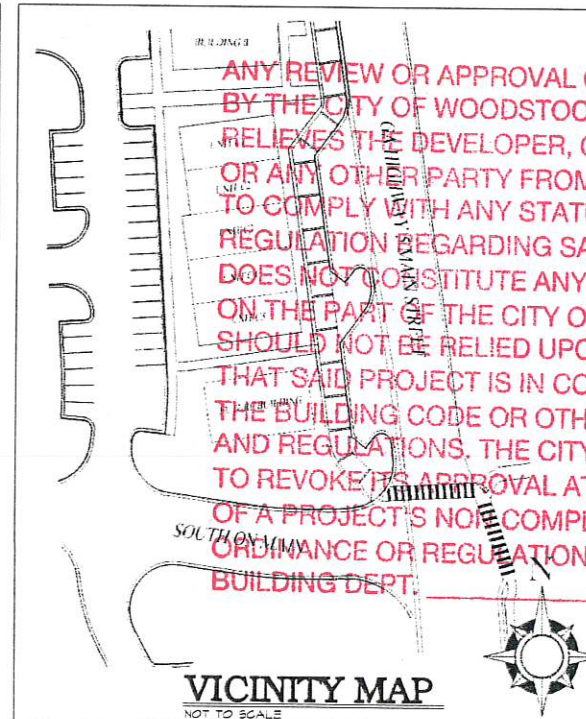
Structural Engineer
Stability Engineering, Inc.
1376 Church Street
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Decatur, Georgia 30030
(404) 377-9316

Mechanical, Electrical, & Plumbing Engineers
Marshall & Bollwerk Engineering, Inc.
8681 Hwy 92, Suite 400
Woodstock, Georgia 30189
(678) 795-0333

Civil Engineer
Rochester & Associates, Inc.
425 Oak Street NW
Gainesville, Georgia 30501
(770) 718-0600

GENERAL NOTES

- All structural design for roofs, floor/ceiling assemblies, and exterior deck structures shall be provided by contractor and shall provide adequate support for all members continuous to foundation in compliance with all applicable code requirements.
- All work shall be performed in accordance with all state and local codes and regulations.
- Contractor shall ensure compatibility of the building with all site requirements.
- Contractors shall verify all conditions and dimensions before beginning construction. Any discrepancies shall be reported to the architect for justification and/or correction before proceeding with the work. Contractors shall assume responsibility for all errors that are not reported.
- All dimensions shall be read or calculated and never scaled.
- Building is designed for a uniform 2000 psf soil design pressure. Contractor to verify adequate soil bearing is provided before proceeding with installation of foundation.



VICINITY MAP
NOT TO SCALE

ANY REVIEW OR APPROVAL OF THIS DOCUMENT BY THE CITY OF WOODSTOCK IN NO WAY RELIEVES THE DEVELOPER, CONTRACTOR, PROPERTY OWNER OR ANY OTHER PARTY FROM THEIR RESPONSIBILITY TO COMPLY WITH ANY STATUTE, ORDINANCE, OR OTHER REGULATION REGARDING SAID PROJECT. THIS REVIEW DOES NOT CONSTITUTE ANY ASSUMPTION OF LIABILITY ON THE PART OF THE CITY OF WOODSTOCK AND SHOULD NOT BE RELIED UPON AS EVIDENCE THAT SAID PROJECT IS IN COMPLIANCE WITH THE BUILDING CODE OR OTHER APPROPRIATE LAWS AND REGULATIONS. THE CITY RESERVES THE RIGHT TO REVOKE ITS APPROVAL AT ANY TIME IT LEARNS OF A PROJECT'S NON-COMPLIANCE WITH ANY ORDINANCE OR REGULATIONS.
BUILDING DEPT. 10-18-21



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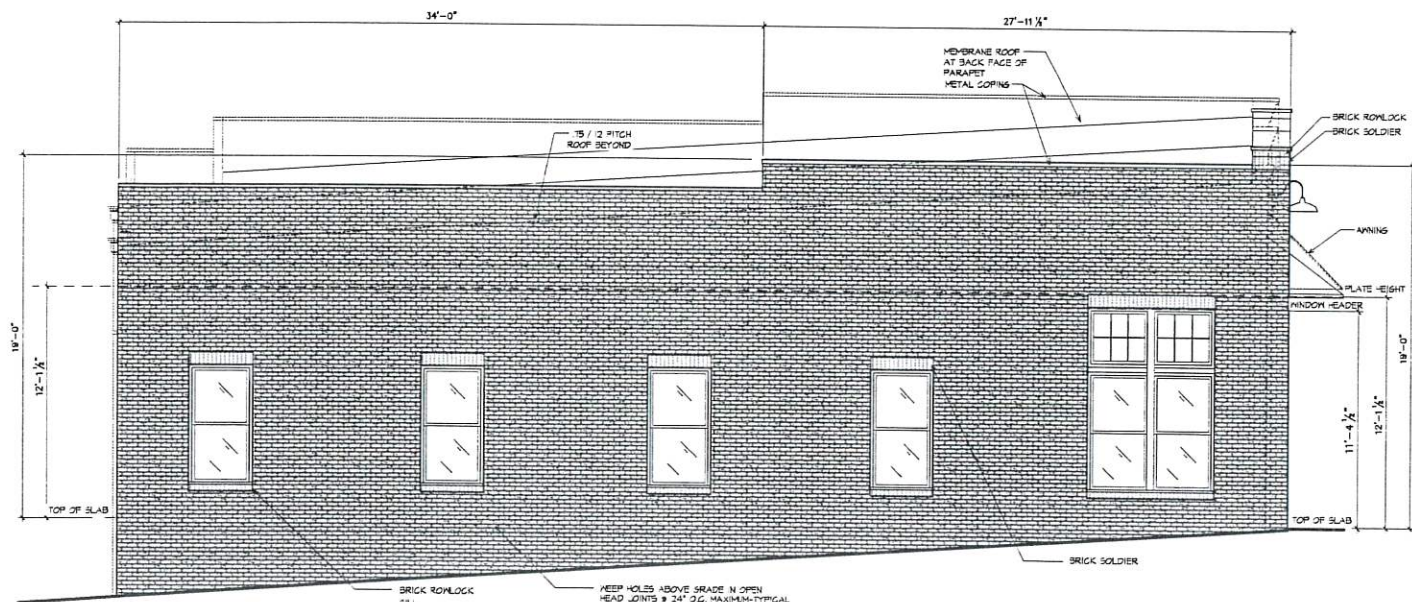
South On Main
Building C
Woodstock, Georgia

REVISIONS		
NO.	DATE	DESC.
1	10-1-21	ISSUED

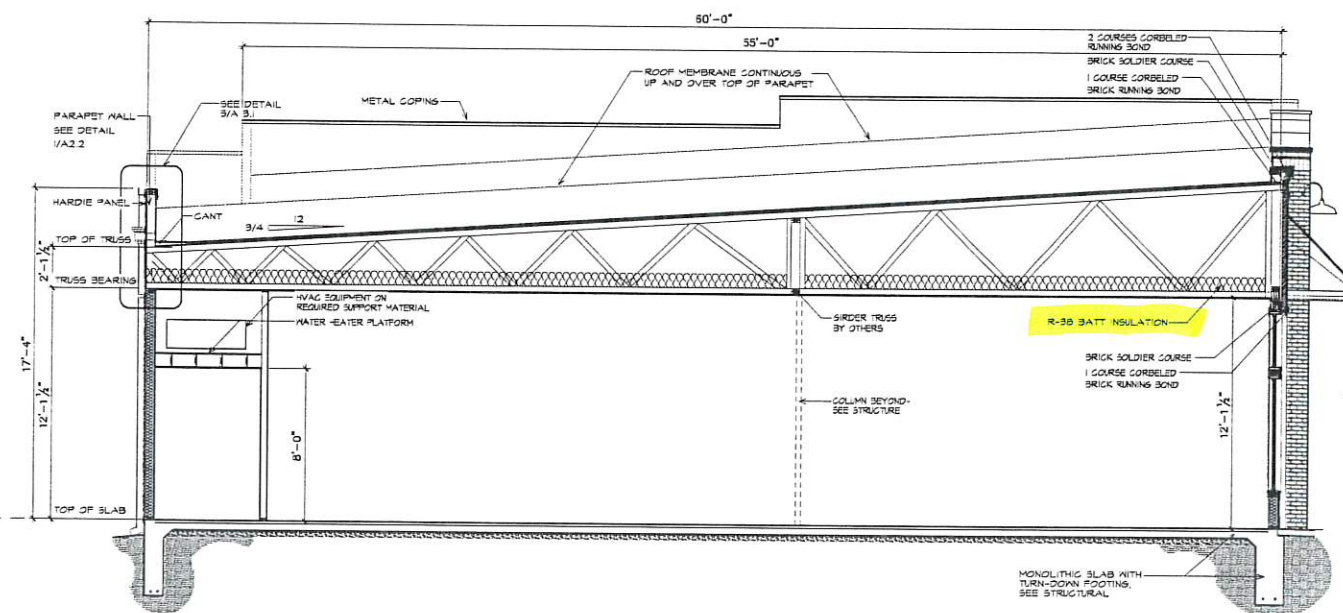
COPYRIGHT © 2021 PEEK DESIGN GROUP, INC.	
DESCRIPTION	COVER SHEET
PROJECT NO.	10-18-21
DATE	10-1-21
CS	

PEEK DESIGN GROUP
Suite 501
3950 Cobb Parkway NW
Acworth, Georgia 30101
Phone: 770.222.4345
www.peekdesign.com

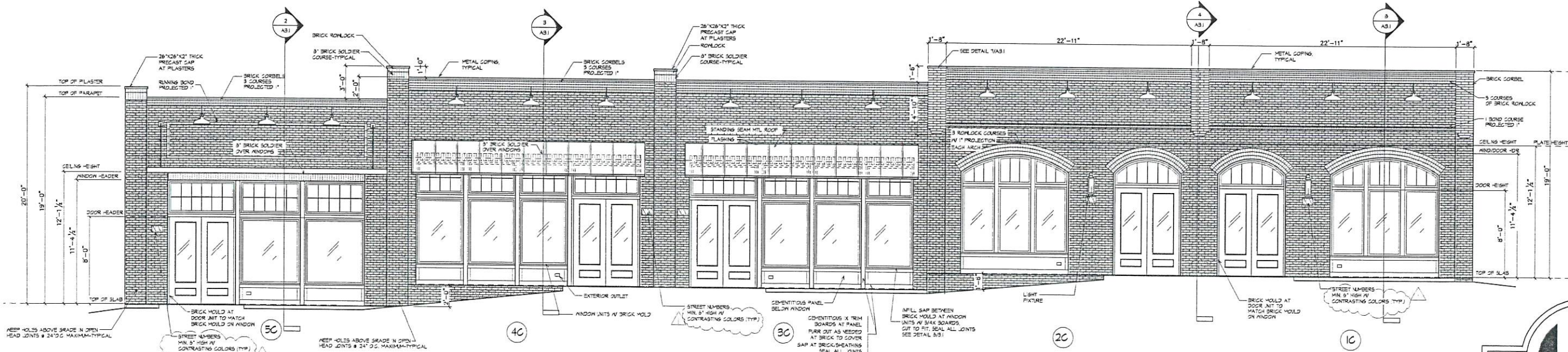
FOR CONSTRUCTION



1 LEFT ELEVATION
A1.1 1/4" = 1'-0"



2 BUILDING SECTION
A1.1 1/4" = 1'-0" (60' DEEP)



3 FRONT ELEVATION
A1.1 1/4" = 1'-0"

FOR CONSTRUCTION

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Smyrna, Georgia 30339



South On Main
Building C
Woodstock, Georgia

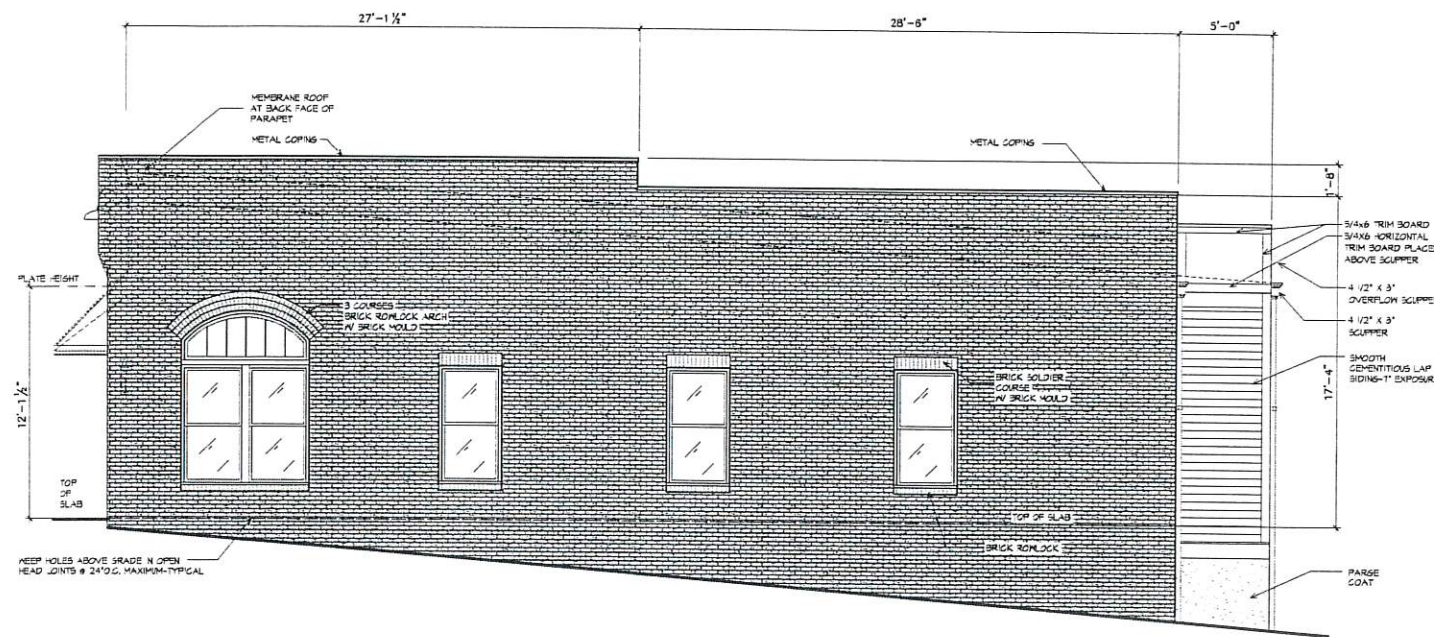
REVISIONS	NO.	DATE	DESCRIPTION
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COMPILED BY	JDC
DESIGNED BY	JDC
PROJECT NO.	2019-001
DATE	04/12/20

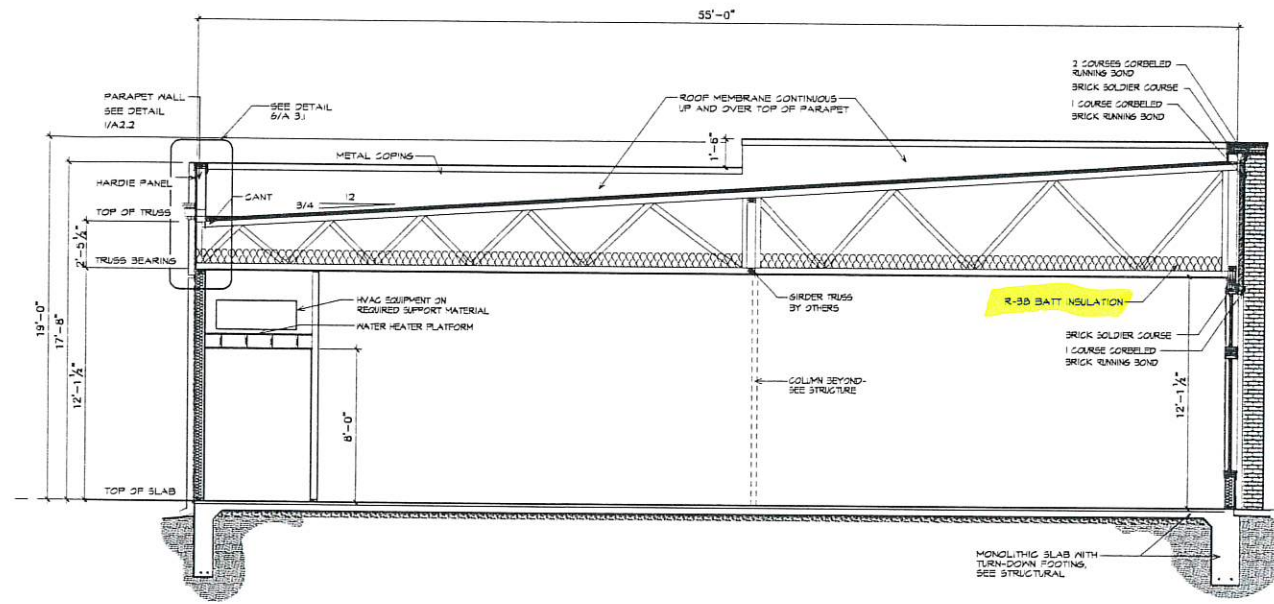
A1.1



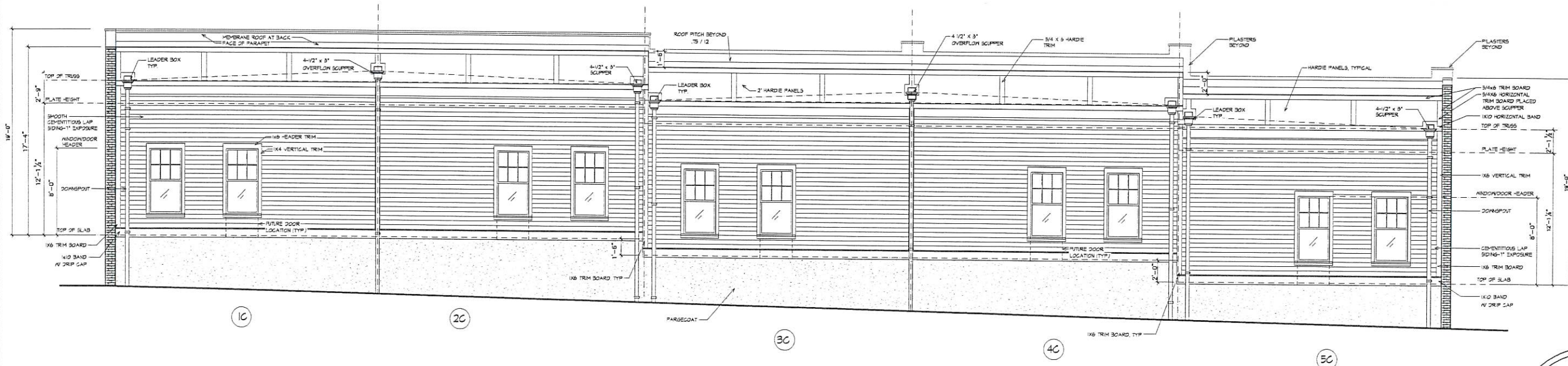
Studio 401
1440 Cobb Parkway NW
Acworth, Georgia 30101
Phone: 770.222.4344
www.peekdesign.com



1 RIGHT ELEVATION
A1.2 1/4" = 1'-0"



2 BUILDING SECTION
A1.2 1/4" = 1'-0" (55' DEEP)



3 BACK ELEVATION
A1.2 1/4" = 1'-0"

FOR CONSTRUCTION

J. W. Collection
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South On Main
Building C
Woodstock, Georgia

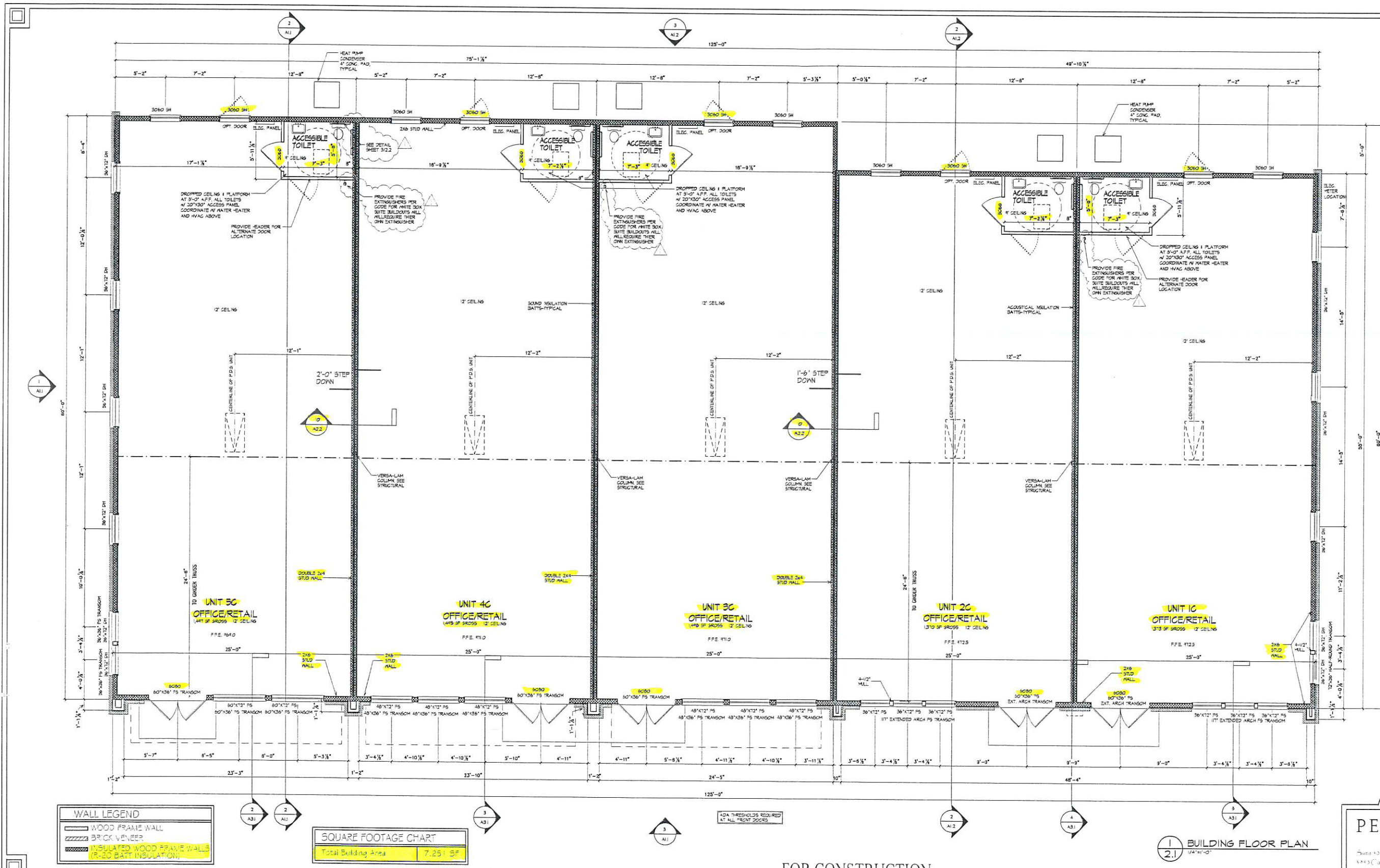
NO.	DATE	DESCRIPTION
1	08/01/2018	PRELIMINARY DESIGN
2	08/01/2018	FINAL DESIGN
3	08/01/2018	FOR CONSTRUCTION

PROJECT NO.	2355
DESCRIPTION	BACK ELEVATION
DATE	08/01/2018
BY	J. W. Collection

A1.2



Phone: 404.333.2222
Address: 1111 Peachtree Parkway, NW, Atlanta, Georgia 30329
Website: www.peekdesign.com



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2355 Log Cabin Drive
Smyrna, Georgia 30339



South On Main
Building C
Woodstock, Georgia

REVISIONS		
NO.	DATE	DESCRIPTION
1	10/1/21	ISSUED FOR PERMITS

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PEEK DESIGN GROUP, INC.	
BUILDING FLOOR PLAN	
PLAN NO.	DATE
A2.1	10/1/21

A2.1

5000 N. 1st St.
Atlanta, Georgia 30309
Phone: 404.222.4444
www.peakdesign.com

FOR CONSTRUCTION

1 GRAB BAR BLOCKING DETAIL
A2.2 N.T.S.

2 SECTION THRU LAVATORY
A2.2 N.T.S.

3 HANDICAP ELEVATIONS
A2.2 N.T.S.

4 TYPICAL JAMB DETAIL
A2.2 SCALE: 1"=1'-0" REAR WINDOWS

5 TYPICAL MULL DETAIL
A2.2 SCALE: 3/4"=1'-0" HORIZONTAL MULL SIMILAR

6 TYPICAL HEAD DETAIL
A2.2 SCALE: 1"=1'-0" REAR WINDOWS

7 TYPICAL SILL DETAIL
A2.2 SCALE: 1"=1'-0" REAR WINDOWS

8 BUILDING ROOF PLAN
A2.2 3/16"=1'-0"

9 CEILING AT STEPPED UNIT
A2.2 CROSS SECTION THRU ROOF SCALE: 1 1/2"=1'-0"

10 TYPICAL SEPARATION WALL
A2.2 SECTION-STEPPED FLOOR SCALE: 1 1/2"=1'-0"

FOR CONSTRUCTION

PEEK DESIGN GROUP
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South On Main Building C
Woodstock, Georgia

J. W. Collection
2355 Log Cabin Drive
Smyrna, Georgia 30339

STATE OF GEORGIA
MARSHALL W. JR.
REGISTERED ARCHITECT

REVISIONS

NO.	DATE	REVISION
1	11/13/11	2/1

DESCRIPTION

BUILDING ROOF PLAN

PLAN NO. 2075
DATE 8/1/11

A2.2

J. W. Collection
2355 Log Cabin Drive
Smyrna, Georgia 30339



South On Main
Building C
Woodstock, Georgia

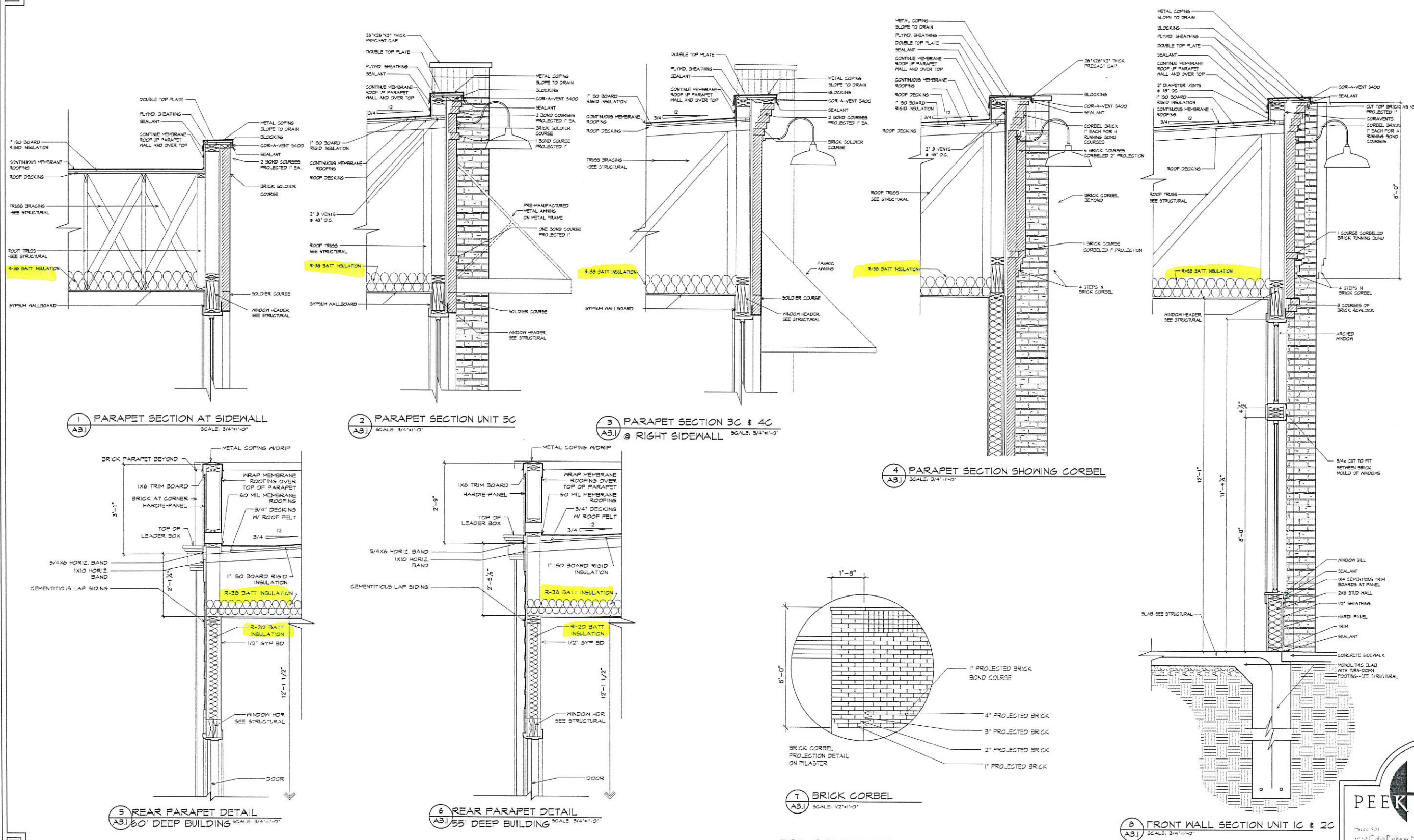
NO.	DATE	DESC.

PROJECT NO.	108
DESIGNER	PEEK DESIGN GROUP, INC.
DATE	8-1-2011

A3.1

PEEK DESIGN GROUP

12000 Peachtree Parkway, NW
Atlanta, Georgia 30329
Phone: 770.222.4444
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FOR CONSTRUCTION

GENERAL NOTES:

1. THE GENERAL CONTRACTOR SHALL COORDINATE ALL MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS, INCLUDING THE SIZE AND LOCATION OF MISCELLANEOUS ITEMS AFFECTING THE STRUCTURAL WORK SUCH AS SMALL OPENINGS, PIPE SLEEVES, RECESSES, BENT PLATES, ETC. PROMPTLY NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES OR OMISSIONS. OPENINGS THROUGH BEAMS, GIRDERS AND/OR COLUMNS SHALL BE VERIFIED BY ENGINEER.
2. THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND EXISTING SITE CONDITIONS PRIOR TO COMMENCING WORK. PROMPTLY NOTIFY THE ARCHITECT/ENGINEER OF ANY DISCREPANCIES BETWEEN ACTUAL SITE CONDITIONS AND THE CONTRACT DOCUMENTS.
3. THE CONTRACTOR SHALL VERIFY ALL FLOOR AND ROOF MOUNTED MECHANICAL EQUIPMENT DIMENSIONS AND WEIGHTS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS AND REVIEWED SHOP DRAWINGS.
4. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. THE ERECTION PROCEDURE AND SEQUENCE INCLUDING THE DESIGN ADEQUACY AND SAFETY OF ERECTION BRACING, SHORING, RE-SHORING, TEMPORARY SUPPORTS, ETC., ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
5. DO NOT SCALE DRAWINGS. ALL WORK REQUIRING MEASURING SHALL BE DONE ACCORDING TO FIGURES ON DRAWING. ANY MISSING DIMENSIONS WILL BE FURNISHED UPON REQUEST.
6. WHERE A DETAIL IS SHOWN FOR ONE CONDITION, IT SHALL ALSO APPLY FOR ALL LIKE OR SIMILAR CONDITIONS UNLESS NOTED OTHERWISE.
7. THESE GENERAL NOTES APPLY WHERE OTHER PROVISIONS ARE NOT PROVIDED BY THE DRAWINGS, SPECIFICATIONS OR TYPICAL DETAILS. IN CASE OF SPECIAL CONDITIONS INDICATED ON DRAWINGS, THE DRAWINGS SHALL GOVERN OVER THE SPECIFICATIONS.
8. THE CONTRACTOR SHALL PROVIDE ALL CENTERLINE-TO-CENTERLINE DIMENSIONS TO THE STEEL FABRICATOR PRIOR TO SHOP DRAWING SUBMITTAL AND FABRICATION OF STRUCTURAL STEEL.
9. CONSTRUCTION SHALL BE IN ACCORDANCE WITH FEDERAL, STATE, AND LOCAL ORDINANCES, AND THE INTERNATIONAL BUILDING CODE 2018.
10. THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING ALL SHORING, BRACING, AND STRUCTURAL SUPPORTS.
11. TEMPORARY SHORING: PROVIDE AND MAINTAIN SHORING, BRACING, AND STRUCTURAL SUPPORTS AS REQUIRED TO PRESERVE STABILITY AND PREVENT MOVEMENT, SETTLEMENT, OR COLLAPSE OF CONSTRUCTION AND FINISHES TO REMAIN AS WELL AS ADJACENT EXISTING STRUCTURES, AND TO PREVENT UNEXPECTED OR UNCONTROLLED MOVEMENT OR COLLAPSE OF CONSTRUCTION BEING DEMOLISHED.

FOUNDATIONS:

1. FOUNDATIONS ARE DESIGNED FOR AN ALLOWABLE SOIL BEARING PRESSURE OF 2500 PSF. THIS VALUE SHALL BE VERIFIED PRIOR TO FOUNDATION CONSTRUCTION. IF ACTUAL VALUES VARY BY MORE THAN TEN PERCENT FROM DESIGN BEARING PRESSURE, FOOTINGS SHALL BE REDESIGNED. ALL FOOTINGS ARE TO BE PLACED ON UNDISTURBED ORIGINAL SOIL OR COMPACTED FILL.
2. ALL BACKFILLING SHALL BE ACCOMPLISHED USING MATERIAL CONSISTING OF CRUSHED STONE AND/OR MATERIAL APPROVED BY A REGISTERED SOILS ENGINEER. FILL MATERIAL TAKEN FROM SITE EXCAVATION SHALL HAVE OPTIMUM MOISTURE CONTENT FOR COMPACTION AND BE FREE OF ANY DEBRIS. BACKFILL SHALL BE COMPACTED TO 95% MAXIMUM DENSITY AS DETERMINED BY ASTM D698. IN MAXIMUM LIFTS OF EIGHT INCHES. NO BACKFILL MATERIAL SHALL BE PLACED AGAINST WALLS WITHOUT PROVISIONS FOR ADEQUATE BRACING OF THESE WALLS.
3. FILL MATERIAL SHALL BE ADEQUATELY DRAINED TO PREVENT ANY DAMAGE TO THE BUILDING'S FOUNDATION BY GROUND WATER FLOWS OR SURFACE WATER RUN-OFF.
4. PROVIDE TERMITE TREATMENT PRIOR TO PLACING VAPOR BARRIER AND CONCRETE SLABS.
5. ALL SOIL CONDITIONS ARE SUBJECT TO EVALUATION BY A SOILS ENGINEER PRIOR TO FOUNDATION CONSTRUCTION. SOILS WITHOUT ADEQUATE CAPACITY TO SUPPORT DESIGN LOADS MUST BE REPLACED OR MODIFIED PRIOR TO PROCEEDING WITH CONSTRUCTION.
6. ALL VAPOR BARRIERS TO BE 10 MIL POLYETHYLENE SHEETING UNLESS NOTED OTHERWISE.
7. PATIO SLABS ARE TO BE 4" THICK STANDARD CONCRETE SLABS WITH TURNED DOWN EDGE.
8. ALL WIRE MESH AND REBARS ARE TO BE PLACED ON SUPPORTS PER ACI STANDARDS PRIOR TO POURING CONCRETE.
9. SEE ARCHITECTURAL DRAWINGS FOR CONCRETE FINISHING REQUIREMENT.
10. ALL CONCRETE RETAINING WALLS SHALL BE CURED AT LEAST SEVEN DAYS AND ADEQUATELY BRACED PRIOR TO BACKFILLING IN ACCORDANCE WITH APPLICABLE CODES.
11. DUE TO POOR SOIL CONDITIONS AT THE BUILDING, HELICAL PIERS ARE REQUIRED FOR MOST FOUNDATION BEARING. HELICAL PIERS ARE TO BE DESIGNED AND INSTALLED BY AN EXPERIENCED PIER CONTRACTOR. PIER DEPTH AND DESIGN CAPACITY SHALL BE VERIFIED DURING INSTALLATION UNDER THE SUPERVISION OF A GEOTECHNICAL ENGINEER.
12. RECOMMENDED PIER TYPE IS 2 1/2" O.D. SCHEDULE 40, HOT DIPPED GALVANIZED.

CONCRETE:

1. CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 301, SPECIFICATION FOR STRUCTURAL CONCRETE, EXCEPT AS NOTIFIED BY THE REQUIREMENTS OF THESE STRUCTURAL DRAWINGS.
2. CONCRETE SHALL BE NORMAL WEIGHT AND HAVE A DESIGNATED COMPRESSIVE STRENGTH (F'_c) IN 28 DAYS OF 3000 PSI WITH A 4" (±1") SLUMP UNLESS NOTED OTHERWISE.
3. SAWN CONTROL JOINTS IN FLOOR SLABS, AS INDICATED ON THE PLANS BY "C.J.", SHALL BE 1/2" DEEP AND SHOULD BE CUT (AS SOON AS CONDITIONS ALLOW) WITHIN 12 HOURS AFTER CONCRETE IS PLACED. MAXIMUM "C.J." SPACING SHALL BE 15 FT IN EITHER DIRECTION.
4. SLAB-ON-GRADE NOT OTHERWISE SPECIFIED, SHALL BE 6" THICK MINIMUM WITH 6x6-W2.34W2.3 WELDED WIRE MESH PLACED 1" FROM TOP OF SLAB. USE PLASTIC MESH "CHAIRS" SPACED SO THAT NO MORE THAN 1 1/2 SQ FEET OF WIRE IS SUPPORTED ON EACH "CHAIR", UNLESS OTHERWISE SPECIFIED. SLAB SHALL BE PLACED ON 5-MIL VAPOR BARRIER ON 4" OF #57 STONE OR COMPACTED SAND OR CRUSHER RUN. VAPOR BARRIER MAY BE OMITTED FROM DRIVES, WALLS, PATIOS, AND OTHER FLAT WORK NOT LIKELY TO BE ENCLOSED AND HEATED AT A LATER DATE. JOINTS LAPPED NOT LESS THAN 6".
5. CONCRETE COARSE AGGREGATE, WITH A MAXIMUM SIZE OF 1" MAY BE USED IN FOUNDATIONS. ALL OTHER CONCRETE SHALL HAVE A COARSE AGGREGATE WITH A MAXIMUM SIZE OF 3/4".
6. CONCRETE MIXING, TRANSPORTING, PLACING, AND CURING SHALL BE DONE IN ACCORDANCE WITH THE RECOMMENDATIONS OF ACI 301. READY-MIXED CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE WITH REQUIREMENTS OF ASTM C94 OR ASTM C885.
7. NO CONSTRUCTION LOADS SHALL BE SUPPORTED ON, NOR ANY SHORING REMOVED FROM, ANY PART OF THE STRUCTURE UNDER CONSTRUCTION EXCEPT WHEN THAT PORTION OF THE STRUCTURE IN COMBINATION WITH REMAINING FOUNDATION AND SHORING SYSTEM HAS SUFFICIENT STRENGTH TO SUPPORT SAFELY ITS WEIGHT AND LOADS PLACED THEREON.
8. THE CLEAR DISTANCE BETWEEN REINFORCING BARS, BUNDLED BARS, PRE-STRESSING TENDONS, AND DUCTS SHALL BE IN ACCORDANCE WITH THE LIMITATIONS OF ACI 318.
9. ALL EQUIPMENT FOR MIXING AND TRANSPORTING CONCRETE SHALL BE CLEAN. ALL DEBRIS, WATER AND ICE SHALL BE REMOVED PRIOR TO PLACING CONCRETE. FORMS SHALL BE PROPERLY COATED. MASONRY FILLER UNITS THAT WILL BE IN CONTACT WITH CONCRETE SHALL BE WELL DRENCHED. REINFORCEMENT SHALL BE CLEAN OF ICE OR OTHER DELETERIOUS COATING. ALL LANTAGE AND OTHER UNSOUND MATERIAL SHALL BE REMOVED BEFORE ADDITIONAL CONCRETE IS PLACED AGAINST HARDENED CONCRETE.
10. MINIMUM COVER FOR CAST-IN-PLACE CONCRETE REINFORCEMENT:

	MINIMUM COVER
(a) CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3
(b) CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 THROUGH #18 BARS	2
#3 BAR, #31 OR #31 WIRE, AND SMALLER	1 1/2
(c) CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
SLAB, WALLS, JOIST:	
#11 BARS AND SMALLER	1 1/2
BEAMS, COLUMNS:	
PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS	1 1/2
11. UNLESS NOTED OTHERWISE, PROVIDE VERTICAL CONTROL JOINTS IN STEP WALLS AND RETAINING WALLS @ 25'-0" O.C. MAXIMUM.

REINFORCING:

1. CONCRETE REINFORCING STEEL SHALL COMPLY WITH THE REQUIREMENTS OF ASTM A615, GRADE 60 FOR #3 BARS AND ASTM A615, GRADE 80 FOR #4 AND LARGER BARS. WELDED WIRE MESH SHALL COMPLY WITH THE REQUIREMENTS OF ASTM A185.
2. DETAILING OF CONCRETE REINFORCEMENT AND ACCESSORIES SHALL BE IN ACCORDANCE WITH ACI 315, LATEST EDITION.
3. REINFORCING STEEL SHALL BE SPLICED ONLY AS INDICATED ON THE PLANS. WHEN SPLICE LENGTHS ARE NOT GIVEN ON THE PLANS, THEY SHALL BE TAKEN FROM THE TABLE BELOW. USE "CLASS B" SPLICES UNLESS THE PLANS INDICATE "CLASS A".

BAR SIZE	CLASS "B" SPLICE CLASS "A" SPLICE
#3	28" 22"
#4	37" 29"
#5	47" 36"
#6	58" 43"
4. HORIZONTAL WALL REINFORCEMENT SHALL BE CONTINUOUS AND HAVE 90-DEGREE BENDS AND EXTENSIONS AT CORNERS AND INTERSECTIONS AS SHOWN ON TYPICAL BAR PLACING DETAILS UNLESS OTHERWISE SHOWN.
5. LAPS IN W.W.F. SHOULD BE ONE MESH PLUS TWO INCHES AT SPLICES.
6. TEMPERATURE STEEL FOR SLABS SHALL BE #3 @ 16" O.C. OR 6x6-W2.34W2.3 W.W.F. PLACED 1" FROM TOP OF SLAB UNLESS SHOWN OTHERWISE. WHERE N.W.F. IS USED, USE PLASTIC MESH "CHAIRS" SPACED SO THAT NO MORE THAN 1 1/2 SQ FEET OF MESH IS SUPPORTED ON EACH "CHAIR", UNLESS OTHERWISE SPECIFIED.
7. ALL BAR HOOKS SHALL BE STANDARD 90-DEGREE HOOKS UNLESS NOTED OTHERWISE ON THE DRAWINGS.
8. SEE CHAPTERS 19 & 20 OF ACI 318-14 FOR CONDITIONS NOT NOTED.
9. DESIGN OF STRUCTURAL ELEMENTS INCLUDING WALLS, FORMED SLABS, BEAMS AND COLUMNS IS IN ACCORDANCE WITH ACI 318

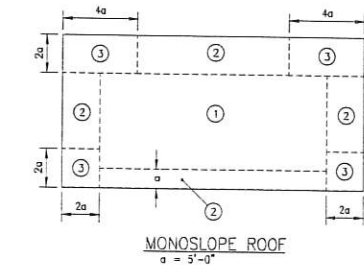
WOOD:

1. ALL CONVENTIONAL TIMBER CONSTRUCTION SHALL CONFORM TO THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" BY THE AMERICAN FOREST AND PAPER ASSOCIATION. ALL STUDS SHALL BE #2 SPRUCE-PINE-FIR OR BETTER. ALL OTHER TIMBER SHALL BE STRUCTURAL GRADED #2 SOUTHERN PINE OR BETTER UNLESS NOTED OTHERWISE. PLYWOOD CONSTRUCTION SHALL UTILIZE AMERICAN PLYWOOD ASSOCIATION RATED MATERIALS.
2. ALL TIMBER IN CONTACT WITH CONCRETE OR MASONRY WITHIN 5" OF GRADE, OR REMAIN EXPOSED TO WEATHER SHALL BE PRESSURE TREATED #2 SOUTHERN PINE, ANPA STANDARD U1.
3. AT LOAD BEARING WALLS, TOP PLATE SHALL BE DOUBLE. SILL PLATE SHALL BE SINGLE. ALL LOAD BEARING WALLS SHALL BE CONSTRUCTED OF STUDS AND PLATES WITH A MOISTURE CONTENT ≤ 19%.
4. ROOF SHEATHING SHALL BE 40/20 19/32" MIN. APA RATED SHEATHING U.N.O. LAM WITH FACE GRAIN PERPENDICULAR TO THE FRAMING AND STAGGERED 4'-0". MINIMUM NAILING SHALL BE 3d NAILS @ 9" O.C. ALL UNSUPPORTED EDGES OF PLYWOOD SHEATHING SHALL BE SUPPORTED WITH SIMPSON PSL CLIPS. PROVIDE (2) CLIPS EQUALLY SPACED BETWEEN EACH TRUSS/SUPPORT.
5. SUB-FLOOR SHEATHING SHALL BE 48/24 1/2" APA RATED PLYWOOD U.N.O. GLUED AND NAILLED TO JOISTS. MINIMUM NAILING SHALL BE 3d NAILS AT 9" O.C. ON PANEL EDGES AND 12" O.C. AT INTERIOR SPLICES.
6. TYPICAL EXTERIOR SHEAR WALL SHEATHING SHALL BE 1/2" OSB U.N.O., FASTENED WITH 3d x 2 1/2" LONG NAILS - SEE 8/5-2.0 FOR SHEAR WALL SHEATHING AND FASTENING SCHEDULE. CONTRACTOR TO ASPECT ALL NAILING BEFORE EXTERIOR VENEER IS PLACED. DRYWALL NAILING TO BE APPROVED BEFORE TAPING. PROVIDE MINIMUM 2x4 @ 16" O.C. STEEL PLATE W/ASHER @ EXTERIOR WALL AND SHEAR PANEL ANCHOR BOLTS. SEE NOTE #13 BELOW FOR TYPICAL BOLT SIZE & SPACING.
7. ALL EXTERIOR WALLS TO BE 2x6 @ 16" O.C. ALL INTERIOR WALLS SHALL BE:

MIN. 2x4 @ 15" O.C.

 SEE WOOD STUD WALL NOTES FOR FURTHER STUD INFORMATION.
8. PLACE (2) STUDS MINIMUM IN WALLS FOR BEARING OF ALL BEAM, HIP, VALLEY MEMBERS U.N.O. AS GREATER ON PLANS. EACH LAYER OF GANGED STUDS SHALL BE NAILED TO ADJACENT STUDS WITH 10d NAILS @ 3" O.C. STAGGERED. CONTINUE ALL POSTS DOWN TO FOUNDATION OR TRANSFER BEAMS BELOW.
9. BEAMS DESIGNATED ON THE PLANS AS LVL SHALL BE VERSA-LAM 2.0E 3100PSI LAMINATED VENEER LUMBER AS MANUFACTURED BY BOISE CASCADE, OR APPROVED ALTERNATE.
10. COLUMNS DESIGNATED AS VERSA-LAM SHALL BE 1.8E 2750PSI AS MANUFACTURED BY BOISE CASCADE, OR APPROVED ALTERNATE.
11. DURING CONSTRUCTION, PROVIDE BRACING FOR FRAMING UNTIL ALL ELEMENTS FOR EXTERIOR SHEAR WALLS AND FLOOR DIAPHRAGMS ARE IN PLACE.
12. UNLESS DETAILED SPECIFICALLY OTHERWISE, USE A METAL CONNECTOR AT ALL BEAM & TRUSS SUPPORTS. BEAM TO BEAM, POST TO BEAM, AND POST TO FLOOR CONNECTIONS. USE SIMPSON PRODUCTS, OR AN APPROVED EQUAL SIZED ACCORDING TO MANUFACTURERS' RECOMMENDATIONS FOR CONNECTION TYPE AND LOADS ENCOUNTERED.
13. PROVIDE 1" ANCHOR BOLT W/ NUT & WASHER AT SILL PLATE, SEE C/S-0.0. PROVIDE (2) A.B. MIN. PER SILL PLATE SEGMENT W/ (1) A.B. LOCATED @ 4' MIN. & 12' MAX. FROM ENDS.
14. ALL WALLS REFERENCED IN THE DRAWINGS (UNLESS NOTED OTHERWISE) ARE COMMON WIRE NAILS TO BE SIZED AS FOLLOWS:

80 = 0.131"
100 = 0.148"
150 = 0.182"



WOOD STRUCTURAL PANEL ROOF SHEATHING + NAILING SCHEDULE			
SHEATHING THICKNESS	NAILS	PANEL LOCATION	ROOF FASTENING ZONE(S)
			FASTENING SCHEDULE (INCHES ON CENTER)
1/2" OR LESS	3d COMMON	PANEL EDGE	6 3d 3d
		PANEL FIELD	6 3d 3d
		PANEL EDGE	6 3d 3d
3/4" OR GREATER	10d COMMON	PANEL FIELD	6 3d 3d

- NOTES:
- (a) EDGE SPACING ALSO APPLIES OVER ROOF FRAMING AT END WALLS.
 - (b) USE RING-SHANK NAILS IN THIS ZONE.
 - (c) ROOF FASTENING ZONES CORRESPOND TO WIND ZONE DIAGRAM SHOWN ON S0.

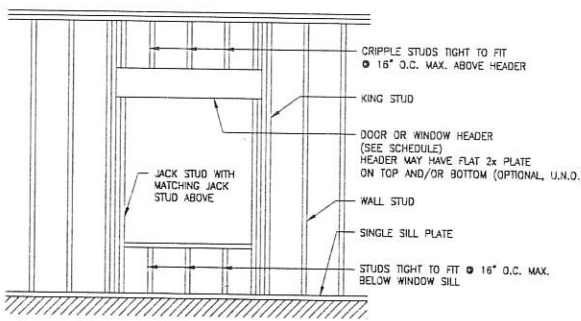
TYP. ROOF FASTENING SCHEDULE

SCALE: N.T.S.

WOOD STUD WALL NOTES:

1. 2x6 STUD MATERIAL:

COMMERCIAL GRADE	#2
SPECIES	SPRUCE-PINE-FIR
GRADING RULES AGENCY	NLA
MOISTURE CONTENT	≤ 19%
(F _b =875 psi; F _c parallel=1150 psi; E=1400 ksi)	
2. MID-POINT BLOCKING: AT FLOOR LOAD BEARING WALLS, A MINIMUM OF (1)2x4 STUD IS REQUIRED MID-WAY BETWEEN FLOOR TRUSSES IN WALLS WHERE TRUSSES ARE PERPENDICULAR TO WALLS. SEE FRAMING PLANS FOR TRUSS ORIENTATION.
3. LOAD BEARING STUD WALLS TO HAVE 2x6 DOUBLE TOP PLATES AND 2x6 SINGLE BOTTOM PLATES. MATERIAL FOR PLATES TO BE NO. 2 SOUTHERN PINE P.T. FOR SILL PLATE IN CONTACT WITH FOUNDATION, AND NO. 2 SPRUCE-PINE-FIR FOR PLATES ABOVE GROUND LEVEL. PROTECT ALL WOOD FRAMING FROM MOISTURE ON THE JOBSITE. IT IS RECOMMENDED THAT ALL HORIZONTAL PLATES HAVE A MOISTURE CONTENT OF 15% OR LESS TO REDUCE SHRINKAGE EFFECTS.
4. NON-LOAD BEARING WALLS THAT ARE NOT SHEAR WALLS SHALL BE UTILITY GRADE SPF OR BETTER @ 24" O.C. MAX. WITH SINGLE UTILITY TOP & SINGLE UTILITY BOTTOM PLATE. SILL PLATE IN CONTACT WITH FOUNDATION SHALL BE NO. 3 SOUTHERN PINE PT.
5. STUDS, STUD POSTS, AND OTHER WOOD POSTS SHOWN WITHIN STUD WALLS ARE DESIGNED WITH THE ASSUMPTION THAT SHEATHING (EITHER GYPSUM OR OSB/PLYWOOD) IS CONTINUOUSLY ATTACHED TO A MINIMUM OF (1) FACE OF EACH STUD OR POST, PERPENDICULAR TO THE PLANE OF THE WALL, PER THE INDICATED ATTACHMENT SPACING IN THE FINAL CONSTRUCTION. THE CONTRACTOR IS TO PROVIDE TEMPORARY BRACING UNTIL ALL ELEMENTS ARE IN PLACE (SEE WOOD NOTES 11/5-0).
6. EACH LAYER OF MULTIPLE STUDS SHALL BE NAILED TO ADJACENT STUDS WITH 10d NAILS @ 3" O.C. STAGGERED. STUD POST SIZE SHALL MATCH STUD WALL SIZE, EXCEPT AS NOTED ON PLAN.
7. CUTTING AND NOTCHING OF ALL LOAD BEARING STUDS SHALL NOT EXCEED A DEPTH OF 25% OF THE STUD MOTH (3/4" FOR 2x4 STUDS). NOTCHES EXCEEDING 25% OF STUD MOTH AND LESS THAN 2x4 SHALL BE REINFORCED WITH A SIMPSON SS STUD SHOE. CONTACT THE STRUCTURAL ENGINEER OF RECORD AT LOCATIONS REQUIRING LARGER NOTCHES. POSTS SHALL NOT BE CUT OR NOTCHED UNLESS SPECIFICALLY NOTED IN THESE DRAWINGS.
8. BORING OF ANY STUD WALL SHALL NOT EXCEED 40% OF STUD MOTH OR BE CLOSER THAN 1/4" FROM EITHER EDGE OF THE STUD. IF A BORED HOLE IS BETWEEN 40% AND 80% OF THE STUD MOTH, THE NUMBER OF STUDS AT THE LOCATION OF THE BORED HOLE SHALL BE DOUBLED AND NO MORE THAN TWO SUCCESSIVE STUD GROUPS SHALL BE BORED. POSTS SHALL NOT BE BORED UNLESS SPECIFICALLY NOTED IN THESE DRAWINGS.



WOOD HEADER SCHEDULE		
SPAN	LOAD BEARING WALLS	NON LOAD BEARING WALLS
H < 3'-0"	(2)2x6	(2)2x6
3'-0" < H < 5'-0"	(2)2x12	(2)2x6
5'-0" < H < 7'-0"	(2)2x12	(2)2x6

JACK/KING STUD SCHEDULE		
STUDS	JACK STUD	KING STUD
(5) GANGED STUDS	(3) STUDS	(2) STUDS
(4) GANGED STUDS	(2) STUDS	(2) STUDS
(3) GANGED STUDS	(2) STUDS	(1) STUD

TYP. FRAMING AT OPENING

SCALE: N.T.S.

COMPONENTS + CLADDING WIND PRESSURES (PSF)				
ENCLOSED STRUCTURE; 's' = 6.7 ft				
A _e (EFFECTIVE AREA)	ROOF PRESSURES			
	ZONE 1	ZONE 2	ZONE 3	
<10 SQ. FT.	+18.0, -25.3	+18.0, -29.3	+18.0, -39.3	
25 SQ. FT.	+18.0, -25.3	+18.0, -29.0	+18.0, -35.1	
50 SQ. FT.	+18.0, -25.3	+18.0, -28.4	+18.0, -31.4	
>100 SQ. FT.	+18.0, -25.3	+18.0, -27.3	+18.0, -27.8	
A _e (EFFECTIVE AREA)	WALL PRESSURES			
	ZONE 4	ZONE 5	ZONE 4P	ZONE 5P
<10 SQ. FT.	+21.8, -23.8	+21.8, -29.0	+21.8, -29.3	+21.8, -39.3
50 SQ. FT.	+19.5, -21.3	+19.5, -24.5	+19.5, -28.4	+19.5, -31.4
200 SQ. FT.	+17.5, -19.4	+17.5, -20.7	+17.5, -27.3	+17.5, -27.8
>300 SQ. FT.	+16.3, -18.1	+16.3, -18.1	+16.3, -27.8	+16.3, -27.8

- NOTES:
1. LOADS AND DIAGRAMMS ARE BASED ON THE IBC 2018 AND ASCE 7-16. PRESSURES SHOWN ARE ULTIMATE LOADS. TO CONVERT TO ALLOWABLE (ASD) LOAD, MULTIPLY VALUES BY 0.6
 2. INTERPOLATION MAY BE UTILIZED FOR EFFECTIVE AREAS THAT OCCUR BETWEEN VALUES SHOWN IN THE TABLE
 3. PLUS AND MINUS SIGN INDICATES THE PRESSURE ACTING TOWARDS AND AWAY FROM THE SURFACES, RESPECTIVELY.
 4. A MINIMUM DEAD LOAD OF 15PSF MAY BE USED FOR NET UPLIFT PRESSURES ON ROOF.

SHOP-FABRICATED WOOD TRUSS NOTES:

1. TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" AND THE "NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION" BY THE TRUSS PLATE INSTITUTE.
2. TRUSS LAYOUT SHOWN IN THESE DRAWINGS IS PROVIDED FOR CONCEPTUAL DESIGN ONLY. THE MANUFACTURER AND DESIGNER SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS, BOTH SIGNED AND SEALED BY A LICENSED STRUCTURAL ENGINEER (STATE OF GEORGIA). SUBMITTALS SHALL BE MADE TO THE ARCHITECT AND ENGINEER FOR REVIEW AND, IF REQUIRED, TO THE LOCAL BUILDING OFFICIAL FOR APPROVAL PRIOR TO FABRICATION. SHOP DRAWINGS SHALL INCLUDE LAYOUT PLAN AND ALL CONNECTORS NOT SPECIFIED IN THESE DRAWINGS. CALCULATIONS SHALL BE BASED ON THE SPECIFIED LOADING CONDITIONS SHOWN HEREIN. THE MANUFACTURER SHALL PROVIDE ALL CONNECTIONS BETWEEN TRUSSES AND LOAD BEARING ELEMENTS. TRUSS FABRICATOR IS TO REVIEW AND APPROVE DIMENSIONS, SHAPES AND DETAILS SHOWN ON SHOP DRAWINGS PRIOR TO SUBMITTAL TO THE ARCHITECT AND ENGINEER FOR REVIEW AND COMMENT.
3. FLOOR TRUSSES SHALL BE DESIGNED FOR ALL LOADS IMPOSED, INCLUDING THOSE FROM LOAD BEARING WALLS ABOVE THAT LOAD THE TRUSSES IN A PARALLEL OR PERPENDICULAR MANNER. MULTI-PLY TRUSSES (ORDER TRUSSES) ARE TO BE PROVIDED WHERE A SINGLE TRUSS CAN NOT ADEQUATELY SUPPORT THE IMPOSED LOADS.
4. MANUFACTURED WOOD ROOF TRUSSES SHALL BE SPACED AT 24" O.C. U.N.O. ON THE PLANS.
5. TRUSS MANUFACTURER AND DESIGNER SHALL PROVIDE HANDERS AND CONNECTORS THAT ARE NOT SPECIFIED ON THE DRAWINGS. CONNECTORS THAT ARE SPECIFIED HEREIN ARE BASED UPON SIMPSON "STRONG TIE" WOOD CONSTRUCTION CONNECTORS. SUBSTITUTIONS MAY BE MADE FOR EQUIVALENT PRODUCTS.
6. ROOF TRUSSES SHALL HAVE CONNECTORS AT EACH END TO RESIST UPLIFT FORCES. CONNECTORS ARE TO BE ATTACHED IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATIONS.
7. TRUSS CHORDS SHALL BE 2x4 MIN AND CONSISTENTLY SIZED THROUGHOUT PROJECT.
8. TRUSS DEFLECTION CRITERIA:

ROOF TRUSSES:	L/240 LIVE LOAD ONLY; L/240 TOTAL LOAD
FLOOR TRUSSES:	L/480 LIVE LOAD ONLY; L/360 TOTAL LOAD

 TRUSS DEFLECTIONS BETWEEN ADJACENT TRUSSES SHALL BE LIMITED TO A TRUSS DEFLECTION DIFFERENTIAL OF 1/4", MAXIMUM.
9. TYPICAL TRUSS DESIGN LOADS (U.N.O.):

FLOOR SYSTEM DEAD LOADS:	15 PSF TCOL
FLOOR SYSTEM LIVE LOADS:	40 PSF TCOL
ROOF SYSTEM DEAD LOADS:	10 PSF TCOL
ROOF SYSTEM LIVE LOADS:	20 PSF TCOL
10. MECHANICAL EQUIPMENT MAY BE PRESENT. THE LOCATION AND LOADS OF RTUs SHALL BE COORDINATED AND ACCOUNTED FOR BY THE TRUSS DESIGNER.
11. ALL REQUIRED BRACING SHALL BE SHOWN ON THE ERECTION PLANS PREPARED BY THE FABRICATOR AND BE IN ACCORDANCE WITH 803I L-08 "THE GUIDE TO 2000 PRACTICE FOR HANDLING, INSTALLING, RESTRAINING AND BRACING OF METAL PLATE CONNECTED WOOD TRUSSES" AND RELATED SUMMARY SHEETS.
12. ALL TRUSS CONNECTOR PLATES SHALL BE MANUFACTURED FROM ASTM A446-72 GRADE 4 GALVANIZED STEEL OF NO LESS THAN 20 GA THICKNESS WITH A MINIMUM YIELD OF 33,000 PSI AND AN ULTIMATE TENSILE STRENGTH OF 45,000 PSI.
13. INSTALLATION OF ALL TRUSSES SHALL BE DONE USING A SPREADER BAR WITH A THREE POINT VERTICAL PICK. CARE SHALL BE USED IN LIFTING TO PREVENT HORIZONTAL BENDING.
14. IMPROPERLY HANDLED OR STORED TRUSSES, AS NOTED ABOVE AND IN THE SPECIFICATIONS, SHALL BE REMOVED FROM THE JOBSITE AND REPLACED AT THE EXPENSE OF THE CONTRACTOR.
15. NON-BEARING PARTITION WALLS SHALL NOT BE FRAMED TIGHT TO THE UNDERSIDE OF JOISTS. PROVIDE TRUSS DEFLECTION CLIP AT PERPENDICULAR WALLS AND DEFLECTION BLOCKING AT PARALLEL WALLS. SEE DETAIL K/S-4.
16. ALL PRE-ENGINEERED TRUSS SHOP DRAWINGS SHALL BE AVAILABLE ON THE JOB SITE DURING THE TIMES OF INSPECTION AND SHALL BEAR CLEAR INDICATION THAT THEY HAVE BEEN REVIEWED BY THE PROJECT STRUCTURAL ENGINEER-OF-RECORD.

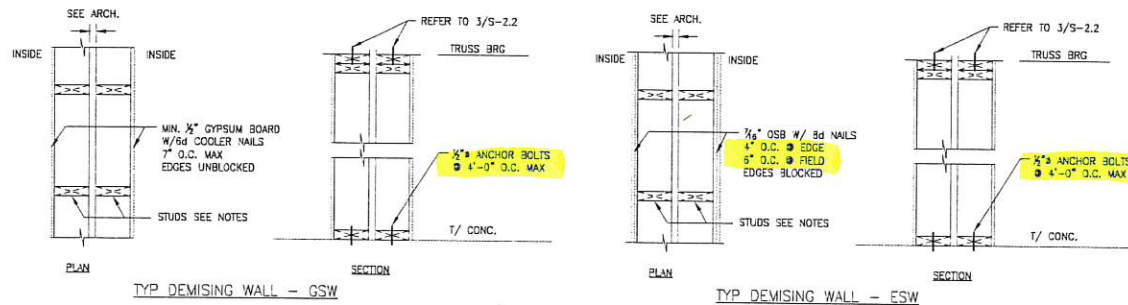
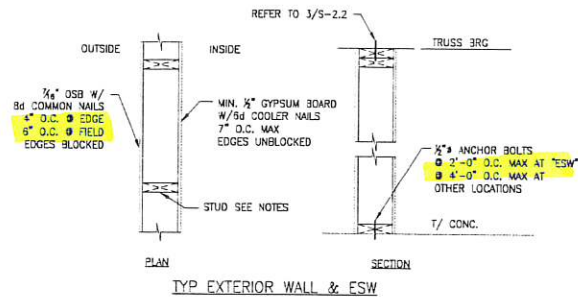
BRICK MASONRY:

1. PROVIDE BRICK VENEER TIES @ 24" O.C. MAX. HORIZONTALLY, 16" O.C. MAX. VERTICALLY. TIES TO BE EITHER:
 - 22GA 1/8" WIDE GALVANIZED CORRUGATED TIES
 - 3GA GALVANIZED WIRE TIES

MASONRY LINTEL SCHEDULE	
CLEAR OPENING	SIZE OF ANGLE
UP TO 3'-0"	1 1/2" x 3/4"
3'-1" TO 7'-0"	1 1/2" x 3/4"
7'-1" TO 9'-0"	1 1/2" x 3/4"
9'-1" TO 10'-0"	1 1/2" x 3/4"
10'-1" AND OVER	1 1/2" x 3/4" ATTACHED TO HEADER W/ 3" x 3" THRU BOLTS @ 12" O.C.

NOTE: LINTELS CARRY MASONRY ONLY. WHERE FLOORS, ROOFS, OR CONCENTRATED LOADS OCCUR, FURTHER ANALYSIS IS NECESSARY. PROVIDE "K" OF BEARING ON EACH END FOR EACH FOOT OF SPAN. MINIMUM BEARING OF 3" ON EACH SIDE OF OPENING IS REQUIRED. USE THIS SCHEDULE UNLESS NOTED OTHERWISE. PAINT LINTEL TO MATCH BRICK.

DESIGN NOTES			
REFERENCED CODES			
INTERNATIONAL BUILDING CODE	IBC	2018	
MINIMUM DESIGN LOADS	ASCE-7	2018	
MASONRY STRUCTURES	ACI 530	2014	
STRUCTURAL CONCRETE	ACI 318	2014	
ASCE	ASCE 360	2016	
LIVE LOADS			
OCCUPANCY OR USE:	UNIFORM (PSF)		
ROOF	20		
PUBLIC OCCUPANCY (COMMERCIAL)	100		
SNOW LOADS			
GROUND SNOW LOAD, P _g		5 PSF	
IMPORTANCE FACTOR, I _s		1.0	
EXPOSURE FACTOR, C _e		0.90	
THERMAL FACTOR, C _t		1.00	
FLAT ROOF SNOW LOAD, P _f		3.15 PSF	
WIND DESIGN CRITERIA			
DESIGN SPEED:		DESIGN BASE SHEAR:	
ULTIMATE (V _u)	105 MPH	(X-X) = 23.3k	
NOMINAL (V _u)	91 MPH	(Y-Y) = 50.1k	
RISK CATEGORY:			
	COMPONENTS & CLADDING WIND	REFER TO TABLES	
	1 PRESSURES:	3/S-0.0	
WIND EXPOSURE (X-X):	9 MWFRS ULTIMATE PRESSURE	16.8 PSF	
WIND EXPOSURE (Y-Y):	3 ULTIMATE ROOF UPLIFT	14.2 PSF	
IMPORTANCE FACTOR (I _w):	1.0	NET SERVICE UPLIFT	0.6*(14.2-10) = 2.3 PSF
INTERNAL PRESSURE COEFFICIENT:	±0.18		
SEISMIC DESIGN CRITERIA			
RISK CATEGORY:			
SEISMIC IMPORTANCE FACTOR (I _s):			1.0
MAPPED SPECTRAL RESPONSE ACCELERATIONS:			S _s = 0.238g S ₁ = 0.092g
SITE CLASS:			D
MAPPED SPECTRAL RESPONSE ACCELERATIONS:			S _s = 0.251g S ₁ = 0.149g
SEISMIC RESPONSE COEFFICIENT (R=6.5):			C _s = 0.0386 C _s = 0.1004
SEISMIC DESIGN CATEGORY:			
SEISMIC-FORCE RESISTING SYSTEM (X-X):	LIGHT-FRAME WALLS SHEATHING WITH WOOD STRUCTURAL PANELS		
RESPONSE MODIFICATION FACTOR (X-X):	R = 6.5		
DESIGN BASE SHEAR (X-X):	F _x = 13.3k		
SEISMIC-FORCE RESISTING SYSTEM (Y-Y):	LIGHT-FRAME WALLS WITH SHEAR PANELS OF OTHER MATERIALS		
RESPONSE MODIFICATION FACTOR (Y-Y):	R = 2.5		
DESIGN BASE SHEAR (Y-Y):	F _y = 35.1k		
ANALYSIS PROCEDURE:	EQUAVENTAL LATERAL FORCE		



A
S-0.1
WALL SHEATHING & ATTACHMENT DETAILS
SCALE: N.T.S.

NOTES:

- SEE ARCH. FOR AIR GAP & FINISH AT OSB SHEATHING AT INTERIOR WALLS.
- 1/2" OSB TO BE APA RATED 24/16 EXPOSURE 1. OSB IS TO BE PROTECTED FROM WEATHER AT ALL TIMES TO PREVENT SWELLING.
- 1/2" ANCHORS TO HAVE 7" MIN. EMBEDMENT. CONTRACTOR OPTION: SUBSTITUTE SIMPSON MASA/MASAP. OR 1/2" SIMPSON TITEN HD W/ 4" EMBEDMENT.
- PROVIDE MIN. (2) ANCHOR BOLTS PER SILL PLATE SEGMENT W/ (1) ANCHOR BOLT LOCATED AT 4" MIN. & 12" MAX. FROM SEGMENT END
- FOR 1/2" ANCHOR BOLTS, PROVIDE STEEL PLATE WASHERS AS FOLLOWS:
2x4 WALL 2 1/2" x 2 1/2" x 1/4" MIN.
2x6 WALL 4 1/2" x 4 1/2" x 1/4" MIN.

MINIMUM REQUIRED SUBMITTALS						
	EPoxy & MECHANICAL ANCHORS	ROOFTOP MECHANICAL EQUIPMENT	CONC. MAX DESIGN AND MATERIALS	WOOD ROOF TRUSSES	STRUCTURAL STEEL & CONNECTIONS	REBAR
PRODUCT LITERATURE	✓	✓	✓			
SHOP DRAWINGS		✓			✓	✓
SHOP DRAWINGS (SIGNED & SEALED)				✓		
CALCULATIONS (SIGNED & SEALED)				✓		

DELEGATED (SPECIALTY) ENGINEERING REQUIREMENTS:

- SEE "MINIMUM REQUIRED SUBMITTALS" TABLE IN THESE DRAWINGS FOR STRUCTURAL ELEMENTS/PRODUCTS WHICH REQUIRE SUBMITTALS.
- DEFINITION OF DELEGATED (SPECIALTY) ENGINEER: A PROFESSIONAL ENGINEER, WHO IS LICENSED IN THE SAME STATE AS THE PROJECT, NOT THE STRUCTURAL ENGINEER OF RECORD, WHO SPECIALIZES IN AND WHO UNDERTAKES THE DESIGN OF STRUCTURAL COMPONENTS PREPARED FOR THIS PROJECT.
- SUBMITTALS FOR CUSTOM DESIGNED, MANUFACTURED OR FABRICATED LOAD CARRYING ITEMS, AND CUSTOM FABRICATED ITEMS THAT ARE REQUIRED BY CODES OR STANDARDS TO RESIST FORCES AND STRESSES, INCLUDING THEIR CONNECTIONS, ANCHORAGES, AND ATTACHMENTS REQUIRE A DELEGATED ENGINEER.
- FOR EACH CATEGORY OF SUBMITTAL REQUIRING INPUT FROM A DELEGATED ENGINEER, THE CONTRACTOR SHALL ATTACH TO THE FIRST SUBMITTAL A SIGNED AND SEALED LETTER FROM THE RESPONSIBLE DELEGATED ENGINEER STATING: "I CERTIFY THAT THE DESIGN AND DRAFTING OF THE SHOP DRAWINGS WHICH ARE SIGNED AND SEALED BY ME WERE PREPARED UNDER MY DIRECT SUPERVISION AND CONTROL, AND TO THE BEST OF MY KNOWLEDGE THE SHOP DRAWINGS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES AND CONTRACT DRAWINGS."
- REVIEW BY THE STRUCTURAL ENGINEER OF RECORD OF SUBMITTALS IS LIMITED TO THE FOLLOWING:
 - THE SPECIFIED STRUCTURAL SUBMITTALS HAVE BEEN FURNISHED;
 - THE STRUCTURAL SUBMITTALS HAVE BEEN SIGNED AND SEALED BY THE DELEGATED ENGINEER;
 - THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND HAS USED THE SPECIFIED STRUCTURAL CRITERIA. NO DETAIL CHECK OF CALCULATIONS WILL BE MADE;
 - THE CONFIGURATION SET FORTH IN THE STRUCTURAL SUBMITTALS IS CONSISTENT WITH THE CONTRACT DOCUMENTS, NOT DETAILED CHECK OF DIMENSIONS OR QUANTITIES WILL BE MADE.
- SUBMITTALS NOT MEETING THE ABOVE CRITERIA WILL NOT BE REVIEWED.

SUBMITTAL NOTES:

- THE GENERAL CONTRACTOR SHALL SUBMIT A SCHEDULE OF SUBMITTALS PRIOR TO CONSTRUCTION BEGINNING ON THE PROJECT. THE SCHEDULE SHOULD DESCRIBE WHAT EACH SUBMITTAL IS, WHETHER IT IS THE ENTIRE PACKAGE, OR BROKEN INTO PHASES FOR REVIEW.
- THE GENERAL CONTRACTOR SHALL ALLOW FOR 10 BUSINESS DAYS OF ALL STRUCTURAL SUBMITTALS. IF THE CONTRACTOR WISHES TO EXPEDITE OR REDUCE THE REVIEW TIME, IT SHALL BE DONE FOR AN ADDITIONAL CHARGE AND MUST BE NEGOTIATED PRIOR TO THE SUBMISSION OF THE PACKAGE.
- IN THE EVENT THAT THE CONTRACTOR ATTEMPTS TO SUBMIT THE ENTIRE PROJECT AT THE SAME TIME FOR SUBMISSION, ADDITIONAL TIME WILL BE REQUIRED BEYOND THE STANDARD 10 DAY REVIEW TIME.

SPECIAL INSPECTION:

- REQUIREMENT IN ACCORDANCE WITH IBC SECTION 1704.3 WILL BE PROVIDED IN SEPARATE DOCUMENTS.
- SPECIAL INSPECTION REPORTS AND FINAL REPORT IN ACCORDANCE WITH SECTION 1704.2.4 SHALL BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO THE TIME THAT PHASE OF WORK IS APPROVED FOR OCCUPANCY.

J. W. Collection
2355 Log Cabin Drive
Smyrna, Georgia 30339



South On Main
Building C
Woodstock, Georgia

REVISIONS	
NO	DATE
1	0-5-2
	STREET

COPYRIGHT 2012
PEEK DESIGN GROUP, INC.
DESCRIPTION:
SHEARWALL
DETAILS

DATE: 9-5-12
S-0.1

**S^e STABILITY
ENGINEERING**
1575 Dunwoody, Suite 200, Dunwoody, GA 30328
SE #21491

ISSUED FOR CONSTRUCTION

**PEEK DESIGN
GROUP**
Suite 401
4442 Cabot Parkway NW
Atlanta, Georgia 30319
Phone: 770.332.4444
www.peakdesign.com

J. W. Collection
2355 Log Cabin Drive
Smyrna, Georgia 30339



South On Main
Building C
Woodstock, Georgia

REVISIONS	
NO.	DATE
1	10-20-20
2	11-03-20

DATE	2020
BY	JWC
CHECKED	JWC
DESCRIPTION	FOUNDATION PLAN
SCALE	1/4" = 1'-0"

S-1.0

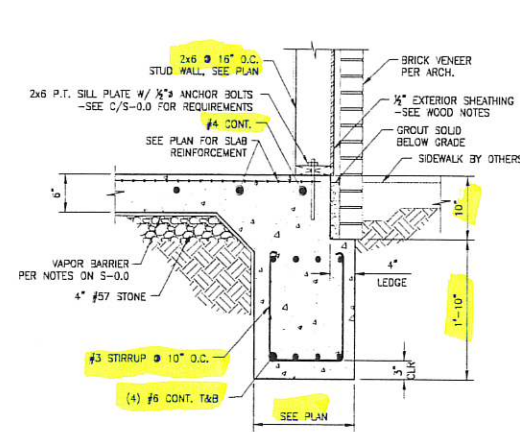
Stability Engineering
SE #21491
ISSUED FOR CONSTRUCTION

PEEK DESIGN GROUP
1475 Peachtree Dunwoody Road, Suite 100
Atlanta, Georgia 30329
Phone: 770.222.4444
www.peakdesign.com

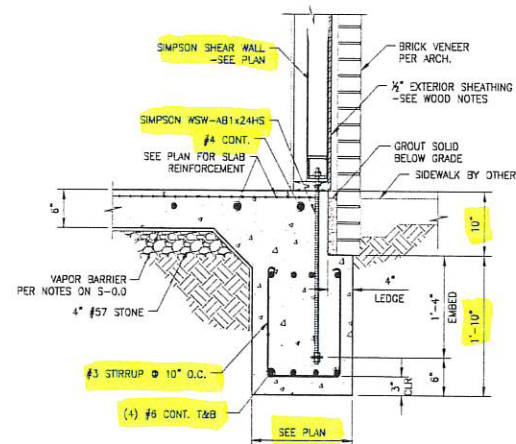
FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

- NOTES:
- FOR GENERAL NOTES, SEE S-0.
 - ALL DIMENSIONS ARE TO F/SLAB, E/SLAB = F/WALL.
 - ALL DIMENSIONS ARE FOR REFERENCES ONLY. SEE ARCH. DRAWINGS FOR DIMENSIONS NOT SHOWN.
 - ARCH. BACKGROUNDS ARE SHOWN FOR GENERAL INFORMATION ONLY. SEE ARCH. FOR EXACT WALL LOCATIONS, DIMENSIONING AND OTHER INFORMATION NOT SHOWN.
 - FOR SHEAR WALL "ESW" & "OSW" AND HOLDOWN SEE DETAILS A, B, & C ON S-0.1.
 - VERIFY DIMENSIONS OF CENTER LINE OF POST WITH ARCH.

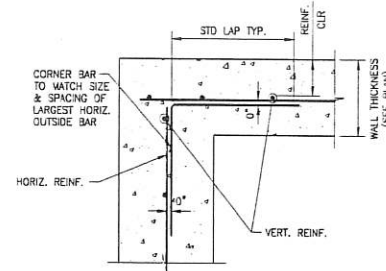
LEGEND	
⊗	POST UP, P3 TYP. U.N.O.
⊙	POST DOWN, P3 TYP. U.N.O.
⊖	POST THRU, P3 TYP. U.N.O.
P2	(2) CHANGED STUDS
P3	(3) CHANGED STUDS
P4	(4) CHANGED STUDS
P5	(5) CHANGED STUDS
CS-2	CONC. WALL
ESW	ESW SHEAR WALL
OSW	OSW SHEAR WALL
SH	SIMPSON HDU2-SOS HOLDOWN



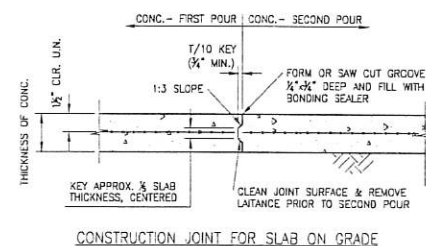
1
S-2.0
FRONT WALL TURNDOWN
WITH CONT. LEDGE
SCALE: 1"=1'-0"



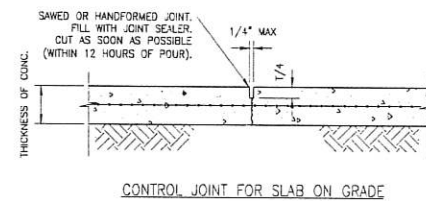
2
S-2.0
SIMPSON WSW HOLDOWN SECTION
SCALE: 1"=1'-0"



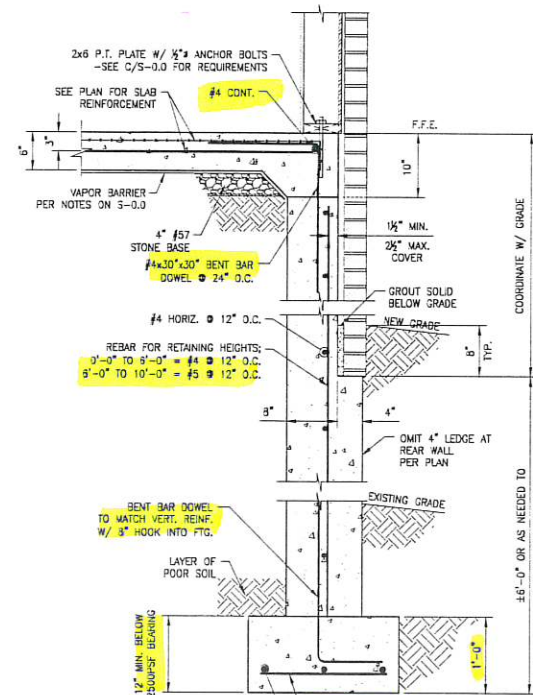
A
S-2.0
WALL REINFORCING AT CORNER
SCALE: N.T.S.



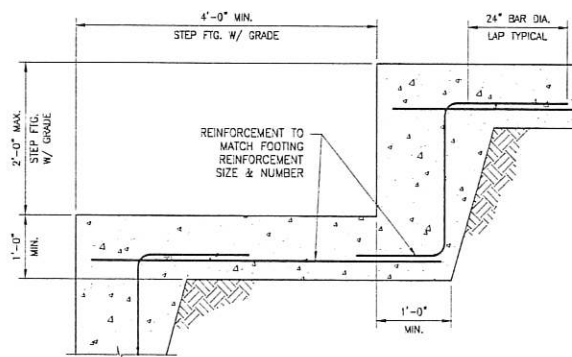
B
S-2.0
CONSTRUCTION JOINT FOR SLAB ON GRADE
SCALE: N.T.S.



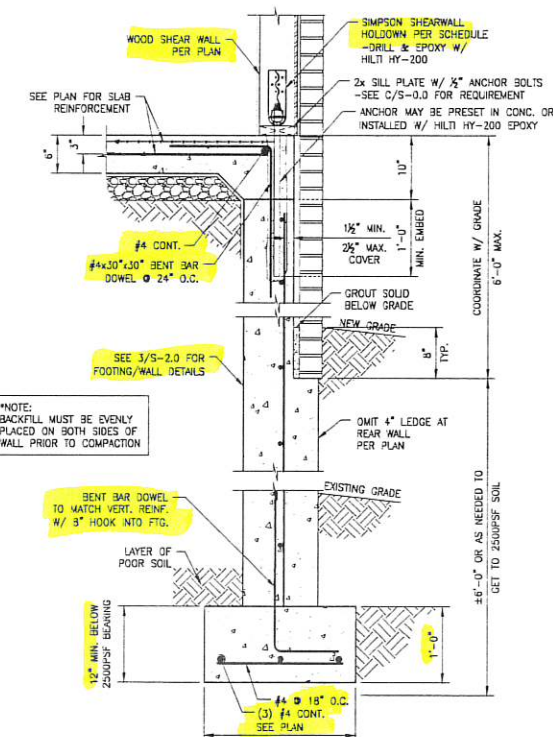
CONTROL JOINT FOR SLAB ON GRADE



3
S-2.0
CONC. STEM WALL
SCALE: 1"=1'-0"



C
S-2.0
STEPPED FOOTING DETAIL
SCALE: 1"=1'-0"



4
S-2.0
STEM WALL @ SHEAR WALL ANCHOR
SCALE: 1"=1'-0"

J. W. Collection
2355 Log Cabin Drive
Smyrna, Georgia 30339



South On Main
Building C
Woodstock, Georgia

REV	DATE	DESCRIPTION
1	05-21	STPLCT

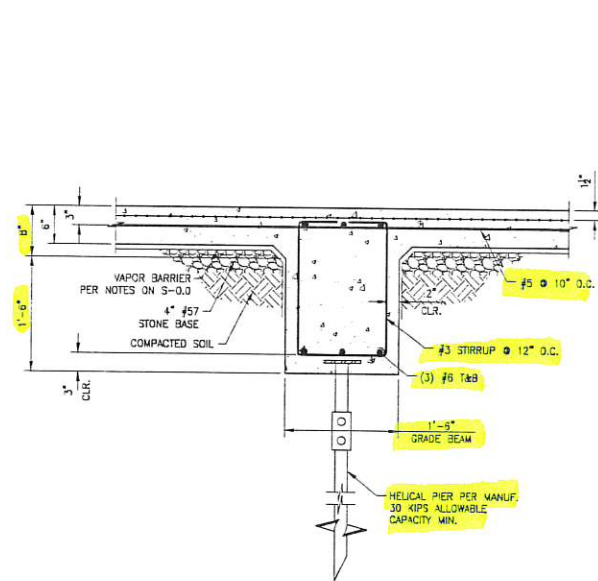
SECTION	DETAILS
SECTION	DETAILS

S-2.0

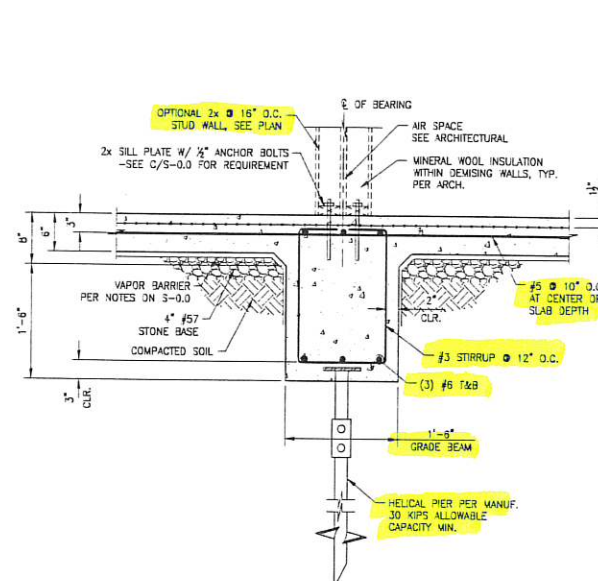
**S^e STABILITY
ENGINEERING**
1075 Cherokee Blvd., Suite 200, Marietta, GA 30066
SE #21491

ISSUED FOR CONSTRUCTION

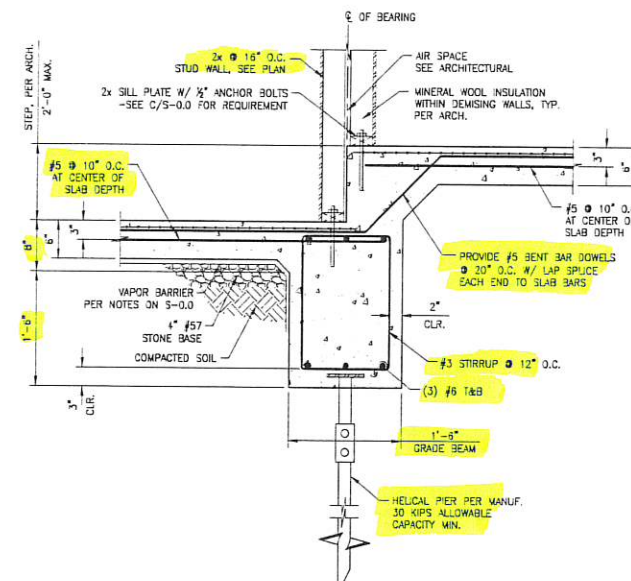
**PEEK DESIGN
GROUP**
1075 Cherokee Blvd., Suite 200, Marietta, GA 30066
Phone: 770.222.4444
www.peakdesign.com



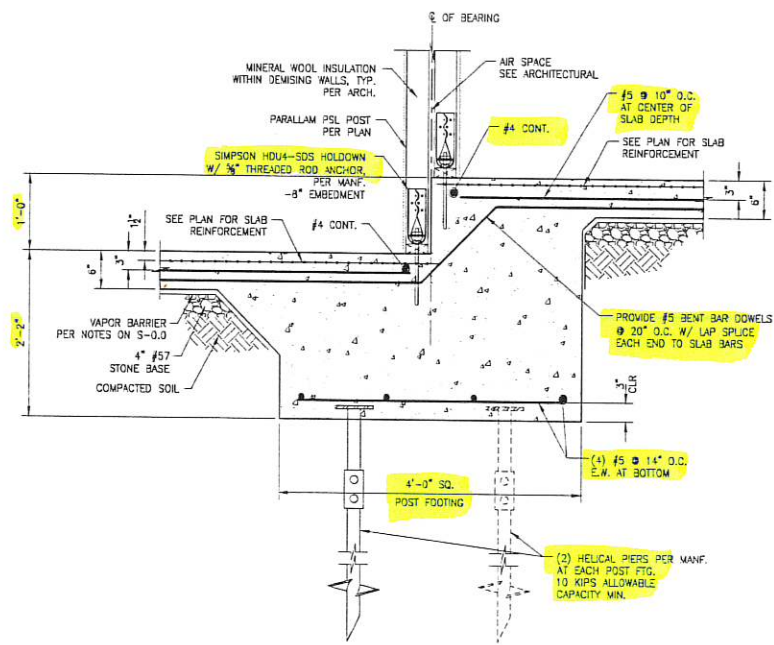
1
S-2.1
TYPICAL GRADE BEAM
W/ HELICAL PIER
SCALE: 1"=1'-0"



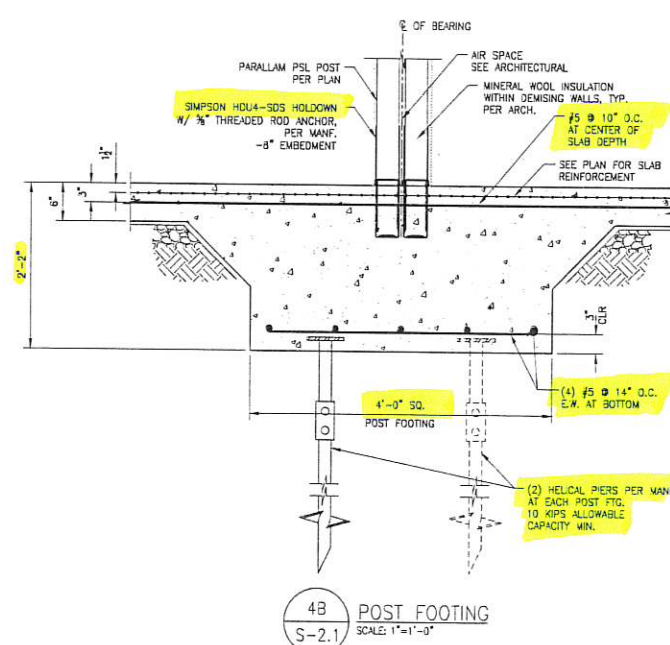
2
S-2.1
GRADE BEAM AT
DEMISING WALL
SCALE: 1"=1'-0"



3
S-2.1
GRADE BEAM AT
STEPPED SLAB
SCALE: 1"=1'-0"



4A
S-2.1
POST FOOTING
SCALE: 1"=1'-0"



4B
S-2.1
POST FOOTING
SCALE: 1"=1'-0"

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Woodstock, Georgia

NO.	DATE	DESC.
1	04-12	STRUCT.

DESIGNED BY	DATE
CHECKED BY	DATE
SECTION	SECTION #
DATE	DATE

S-2.1

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ENGINEERING**
1910 CUMMINS BLVD. SUITE 100, DUBLIN, GA 31033
SE #21491
ISSUED FOR CONSTRUCTION

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